Picton Town Centre Transport Plan 2026

Traffic Analysis and Strategic Design

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Prepared for Wollondilly Shire Council

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Executive Summary

Picton town centre currently experiences traffic congestion in a few key locations, which is forecast to worsen in future years. The strategic context of the town, the surrounding topography and the existing transport networks result in a large proportion of through traffic travelling via the town centre. This through traffic is, in the main, travelling from Tahmoor, Thirlmere and smaller towns and villages to the south of Picton towards the Hume Motorway and Wollongong in the east. Due to a weight restriction on Prince Street, which provides the only other east-west connection, a high volume of heavy vehicles make up the traffic travelling through Picton, affecting pedestrian amenity, safety and the general attractiveness of the town centre.

Considering the historical traffic concerns, and the prospect of large scale land development exacerbating these issues in future, Wollondilly Shire Council developed the Picton Town Centre Transport Masterplan in 2017. This plan identified a number of deficiencies in the transport network and various road infrastructure mitigation measures, including several major intersection upgrades requiring property acquisition.

Subsequent to the masterplan being developed, Council commenced discussions with RMS and private developers regarding a southern bypass of Picton, connecting the Hume Motorway with the Old Hume Highway and funded in large part by development in Wilton Junction and the Greater Macarthur region. This bypass would effectively relieve Picton town centre and provide a more direct route for heavy vehicles to access the state road network. Acknowledging that the Picton Bypass may take 10 years or more to be delivered, Wollondilly Shire Council engaged Cardno to identify interim intersection upgrades in Picton town centre. Cardno's brief was to focus on low-cost, "quick-win" projects that could be delivered relatively quickly and at low cost, while avoiding the potential for redundant infrastructure when the bypass is delivered.

The following report details the design investigations undertaken by Cardno so far. Various upgrade proposals have been modelled using the Aimsun microsimulation modelling package to verify traffic performance and holistic network impacts. A summary of the recommendations from the study is shown in **Table 4-1**. Following on from the traffic modelling investigations, strategic designs were developed for several intersections within the study area. The main focus of the design effort was Prince Street, which is shown to require improvements at both Argyle Street and Menangle Street intersections before 2026 to keep the network running smoothly. In parallel to the traffic modelling and design work, Cardno also undertook a Road Safety Audit of existing conditions at particular locations of interest within the study area. The findings of the audit are detailed in **Appendix C**.

The collective recommendations arising from the traffic modelling, design and road safety investigations will be collated to form the 'Picton Town Centre Transport Plan 2026'. The next step is to discuss this plan with elected members and the wider community, with a view to securing support and funding to allow delivery of the plan by 2026.

1 Introduction

1.1 Background

The Wollondilly Local Government Area is set for significant population growth in the next 20-30 years. These demographic changes are likely to exacerbate traffic issues in Picton, which is a historic town with a constrained road network not equipped to carry large volumes of traffic. In response to this, the Picton Town Centre Transport Master Plan (TDG, 2017) proposed numerous road upgrades, including the replacement of an existing one lane bridge at Prince Street, to improve traffic conditions in the town centre.

Cardno understands the TDG proposals will require long term planning and funding arrangements in order to be implemented in full, due to the large extent of works and private property acquisition required. It is also likely the Picton Town Centre Transport Master Plan will need to be revised due to re-zoning of land and large scale residential development in the Greater Macarthur region.

In the interim, Council commissioned Cardno to identify short term network capacity improvements, essentially quick win schemes that can be implemented within 0-5 years, and then develop designs sufficient for strategic cost estimates and consultation with relevant stakeholders. Collectively these measures are to be known as the 'Picton Town Centre Transport Plan 2026'.



1.2 Scope of Work

The scope of work for this transport plan includes:

- > Familiarisation
 - Review of Picton Town Centre Transport Master Plan and other relevant documents.
 - Review and update of existing Aimsun traffic models for Picton town centre.
- > Traffic Analysis
 - Development of preferred short term improvement options in locations identified by Council.
 - Verify future network performance and design life of proposed short term upgrades using Aimsun and SIDRA modelling.
- > Design Drawings
 - Prepare strategic (pre-concept) design drawings for intersection / road improvements.
 - Determine at a high level the extent of civil works and impact on utilities.
 - Determine approximate land acquisition.
- > Cost estimation
 - Prepare strategic cost estimates based on design drawings.
- > Road Safety Audit
 - Conduct existing conditions road safety audit to determine current issues and road safety concerns.
 - Conduct pre-construction road safety audit of design drawings to determine suitability of proposed upgrades.

1.3 Reference Documents

- > Future Network Deficiency Assessment (TDG, 2015)
- > Planning Proposal Reports
 - Abbotsford
 - Argyle St Business Lands
 - Mushroom Tunnel
 - Picton East
 - Stonequarry Commercial
- > Picton Town Centre Microsimulation Model Development Technical Note (TDG, 2016)
- > Picton Town Centre Transport Master Plan (TDG, 2017)
- > Roads and Maritime Services Traffic Modelling Guidelines (RMS, 2013).

1.4 Report Structure

- > Section 1 introduces the background and objectives of the transport plan
- > Section 2 details the traffic modelling undertaken
- > Section 3 presents the strategic design development and cost estimation
- > Section 4 summarises the recommendations and conclusions.

2 Traffic Modelling

2.1 Base Model Development

Cardo adopted the Aimsun microsimulation models developed by TDG for the Picton Town Centre Transport Master Plan (September 2017, TDG) as a starting point for this transport plan. The Aimsun models cover the study area and include the following key intersections identified as a priority to assess:

- > Menangle Street / Argyle Street
- > Argyle Street / Margaret Street
- > Prince Street / Menangle Street
- > Prince Street / Argyle Street
- > Argyle Street / Barkers Lodge Road
- > Argyle Street / Lumsdaine Street.

Cardno reviewed the models and highlighted the need to amend and update the base model, primarily because the model was not stable. The following sections only summarise the recalibration and revalidation of the base model. Further details of the base model are documented in the Picton Town Centre Microsimulation Model Development Technical Note (December 2016, TDG).

2.1.1 Recalibration

Table 1-1

The base model was calibrated to turn counts and all modelled intersection record GEH < 5. The R^2 values for light vehicles (LV) are within modelling guidelines but heavy vehicles (HV) fall outside of the criteria. However, this is typical when HV numbers are very low and thus more variance is expected. The model was not calibrated to core area standards (as prescribed in the RMS modelling guidelines) due to a lack of comprehensive survey data for the study area.

RMS Modellin	g Guidelines	AM (080	0-0900)	PM (160	0-1700)
Measure	Criteria	LV	HV	LV	HV
GEH < 5	> 85%	100%	100%	100%	100%
GEH < 10	100%	100%	100%	100%	100%
R ²	>0.9	0.98	0.90	0.99	0.88

Table 1-1 summarises the base model calibration results.

Base model calibration results

2.1.2 Revalidation

2.1.2.1 Travel Time

All travel time surveyed routes are within the one minute tolerance limit as prescribed in the RMS modelling guidelines. Travel time survey data was not available for the weekend modelling period, hence Cardno only updated the weekday peak hour models. Further details of the base model are documented in the Picton Town Centre Microsimulation Model Development Technical Note (December 2016, TDG).

Table 1-2 summarises the base model travel time validation results.

Travel time		AM (0700-0900)		PN	1 (1600-1800)	
	Observed	Modelled	Difference	Observed	Modelled	Difference
Route 1 NB	231	184.6	-46.4	191	210.2	19.2
Route 1 SB	163	155.8	-7.2	152	152.5	0.5
Route 2 NB	108	124.4	16.4	108	117.9	9.9
Route 2 SB	93	115.5	22.5	111	115.5	4.5
Route 3 EB	83	97.9	14.9	88	97.7	9.7
Route 3 WB	69	75.1	6.1	98	84.8	-13.2

Table 1-2 Base model travel time validation results

2.1.3 Model Stability

The base models for each peak period are stable as shown in Figure 2-1 and Figure 2-2 below. Statistical analysis shows the model is stable enough and less than 5 random seeds are required.

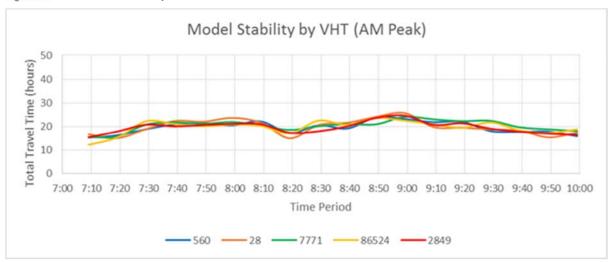
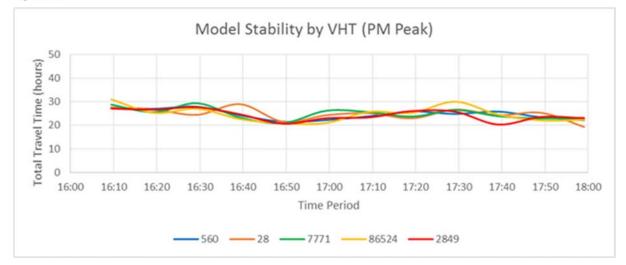


Figure 2-1 Base model stability AM

Figure 2-2 Base model stability PM





2.1.4 Conclusion

Cardo adopted and updated the Aimsun microsimulation models developed by TDG for the Picton Town Centre Transport Master Plan (September 2017, TDG). In summary, the updated base models have:

- > All turn counts GEH < 5
- > High regression value with R² > 0.9 for LVs
- > Stable with less than 5 seed runs required
- > All travel times within 1 minute.

The base models are considered to be stable, robust and fit for the purpose of future year option testing.

2.2 Future Year 2026 Modelling

Cardno developed the future year models (including demand estimation) by adopting the same future year modelling assumptions used in the Aimsun microsimulation models developed by TDG for the Picton Town Centre Transport Master Plan (September 2017, TDG):

> Key developments included in the 2026 model include:

- Abbotsford, located just to the northwest of Picton with access off Bakers Lodge Road
- Mushroom Tunnel, located to the west of Picton with access off the Argyle Street / Lumsdaine Street intersection
- Picton East, located to the east of Picton with access from Margaret Street, Baxters Lane and a new connection on Menangle Street
- Stonequarry Commercial, located to the west of Picton town centre with access onto Elizabeth Street
- > Background traffic growth
 - Wollondilly Growth Management Strategy model used to obtain relative change in trip numbers.

Further details of the demand estimation are documented in the Picton Town Centre Microsimulation Model Development Technical Note (December 2016, TDG). In summary, Cardno estimated 2026 demand by adding the relative change in demand between the 2016 and 2026 models developed by TDG to the Cardno 2016 base matrix as shown in Figure 2-3. This methodology ensures the absolute growth in demand from the above future year modelling assumptions by TDG are reflected in the Cardno Aimsun models.

Figure 2-3 Demand estimation for future year 2026



2.2.2 Future Year Base Conditions

This section reports on the future year 2026 base modelling results. The results are an estimate of future year road network conditions and establishes a future baseline scenario for comparative assessment of options.

2.2.2.1 Menangle Street / Argyle Street

The future year base models indicate during the AM peak, there is a slow moving queue on the southern approach (Argyle Street), due to the short right turn bay and the pedestrian crossings before and after the intersection on Argyle Street. Further delay and queues are observed on the eastern approach (Menangle Street) due to the difficulty in finding gaps, especially for the through and right turners who give priority to the southern and northern approaches. The queue on the eastern approach is much worse during the PM peak.

Model snapshots of these observations are presented in Figure 2-4 and Figure 2-5.



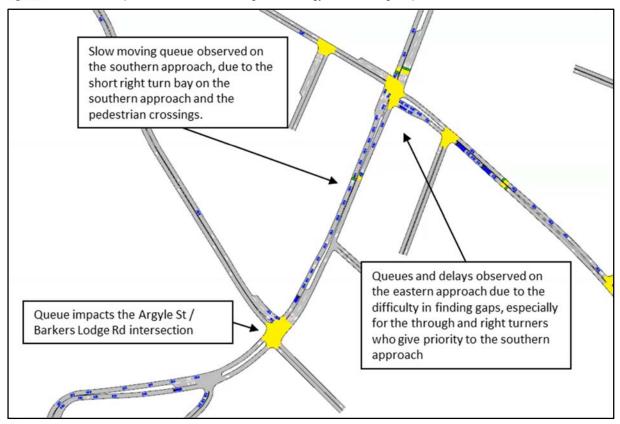
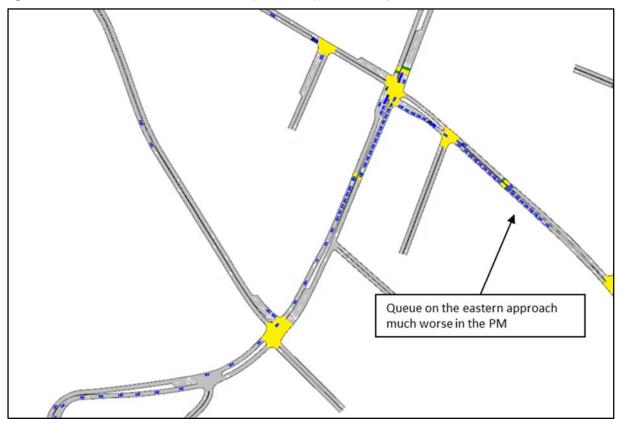


Figure 2-4 Model snapshots of future base Menangle Street / Argyle Street during AM peak

Figure 2-5 Model snapshots of future base Menangle Street / Argyle Street during PM peak





The intersection performance of the Menangle Street / Argyle Street intersection is:

- > LOS D during the AM peak
- > LOS F during the PM peak.

2.2.2.2 Argyle Street / Margaret Street / Cliffe Street

The future year base models indicate no significant issues are anticipated for the Argyle Street / Margaret Street / Cliffe Street intersection.

The intersection performance of the Argyle Street / Margaret Street intersection is:

- > LOS D during the AM peak
- > LOS D during the PM peak.

2.2.2.3 Prince Street / Menangle Street

The future year base models indicate during the AM and PM peak, there are queue and delays observed on the western approach (Prince Street) due to the difficulty in finding gaps, especially for the right turners who give priority to the southern and northern approaches (Menangle Street). The queue on Prince Street can extend to the bridge which is problematic because the bridge is one lane, two way.

Model snapshots of these observations are presented in Figure 2-6 and Figure 2-7.

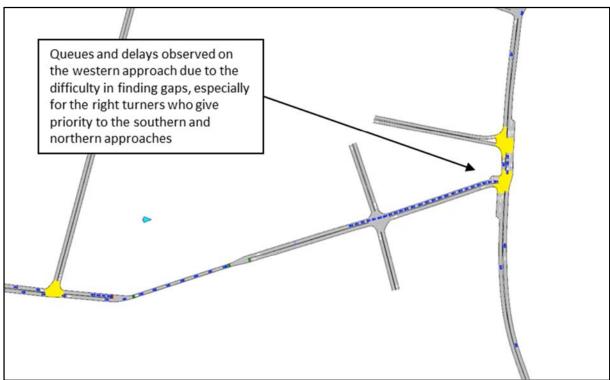
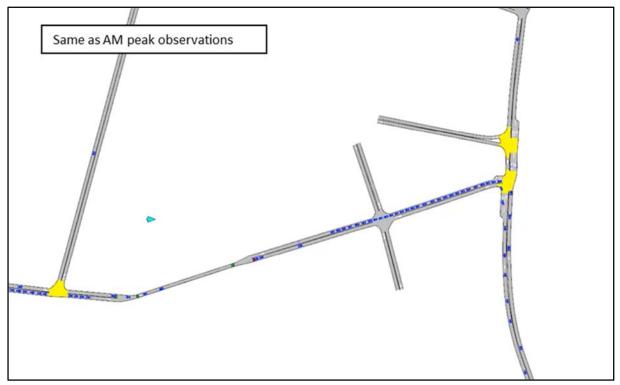


Figure 2-6 Model snapshots of future base Prince Street / Menangle Street during AM peak







The intersection performance of the Prince Street / Menangle Street intersection is:

- > LOS F during the AM peak
- > LOS F during the PM peak.

2.2.2.4 Prince Street / Argyle Street

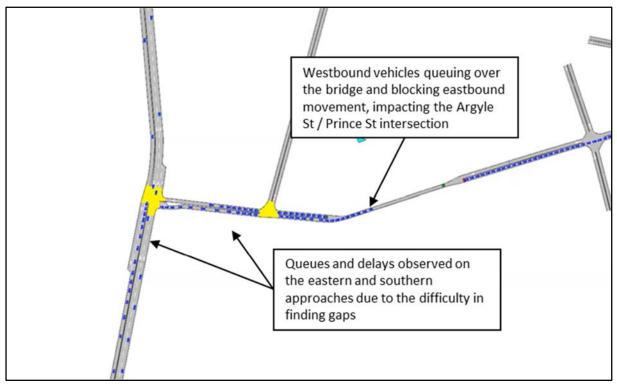
The future year base models indicate during the PM peak, there are queue and delays observed on the eastern approach (Prince Street) and southern approach (Argyle Street) due to the difficulty for vehicles turning to find gaps in traffic. The queue on Prince Street can extend to the bridge which is problematic because the bridge is one lane, two way.

There were no significant issues observed during the AM peak at this intersection.

Model snapshots of these observations are presented below in Figure 2-8.







The intersection performance of the Prince Street / Menangle Street intersection is:

- > LOS B during the AM peak
- > LOS E during the PM peak.

2.2.2.5 Argyle Street / Barkers Lodge Road

The future year base models indicate during the AM and PM peak, there are queue and delays observed on the western approach (Barkers Lodge Road) due to the queuing on Argyle Street originating from the Menangle Street / Argyle Street intersection.

Model snapshots of these observations are presented below in Figure 2-9 and Figure 2-10.



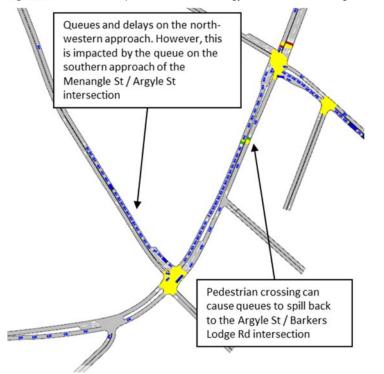
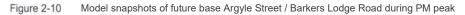
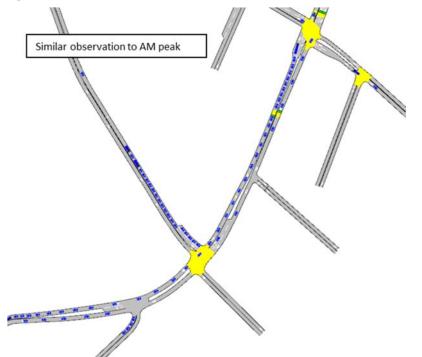


Figure 2-9 Model snapshots of future base Argyle Street / Barkers Lodge Road during AM peak





The intersection performance of the Argyle Street / Barkers Lodge Road intersection is:

- > LOS F during the AM peak
- > LOS E during the PM peak.



2.2.2.6 Argyle Street / Lumsdaine Street

The future year base models show there are no significant issues expected during the AM and PM peaks. However, upgrades are being considered at this intersection to improve pedestrian safety and connectivity, particularly across Argyle Street.

The intersection performance of the Argyle Street / Barkers Lodge Road intersection is:

- > LOS A* during the AM peak
- > LOS A* during the PM peak.

The eastern approach (Lumsdaine Street) was LOS C during both the AM and PM peaks, however the volume for this approach is less than 20 vph during both the AM and PM peaks. The average delay of the next worst approach was adopted for the intersection performance as recommended in the RMS Traffic Modelling Guidelines.

2.2.3 Options

This section describes and comparatively assesses the options proposed for each intersection to address the capacity issues identified in the future base conditions.

2.2.3.1 Menangle Street / Argyle Street

The proposed option for this intersection is a right turn ban on the eastern approach (Menangle Street) as pictured in Figure 2-11. This improves the intersection performance by removing the right turn movement, which incurs the highest delay of the intersection and blocks left turning vehicles out of the eastern approach.

Table 2-1 presents the intersection performance results. The results indicate the proposed option reduces the average delay on the eastern approach from a LOS D to a LOS C in the AM peak, and LOS F to a LOS D in the PM peak when compared to the base case in 2026.



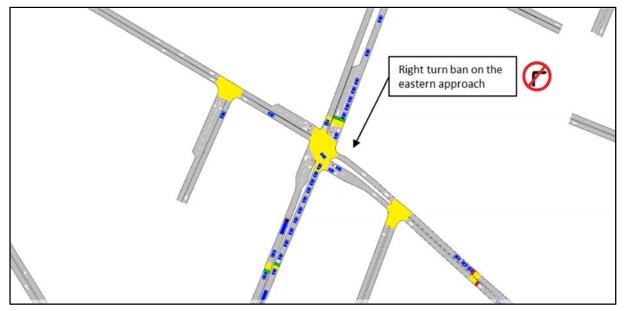


Table 2-1 Intersection Performance Menangle Street / Argyle Street

	AM	РМ
Future Year 2026 Base	LOS D	LOS F
Future Year 2026 Option	LOS C	LOS D

The right turn ban impacts about 30 vph during the AM peak and 27 vph during the PM peak. Since vehicles are rerouted to the Argyle Street / Margaret Street / Cliffe Street intersection, the impact to Argyle Street / Margaret Street / Cliffe Street intersection was also assessed.



Table 2-2 presents the intersection performance results. The results indicate with minor adjustments to the signal control plan (maximum green time increased from 15s to 20s for the eastern approach), the proposed option will have minimal impact to this intersection.

Table 2-2 Intersection Performance Argyle Street / Margaret Street / Cliffe Street			/ Cliffe Street	
		AM	РМ	
Future Ye	ar 2026 Base	LOS D	LOS D	
Future Ye	ar 2026 Option	LOS D	LOS D	

2.2.3.2 Argyle Street / Margaret Street / Cliffe Street

Aside from minor adjustments to the signal control plan proposed as part of the Menangle Street / Argyle Street option, no further upgrades have been proposed for this intersection because no significant issues were identified in the future year base conditions.

2.2.3.3 Prince Street / Menangle Street

The modelling for this intersection assumed the implementation of the upgrade proposed for Menangle Street / Argyle Street, discussed above.

2.2.3.3.1 Option 1

The proposed option for this intersection is signalising the intersection as pictured in Figure 2-12. This is shown to improve intersection performance by being able to control the priority of movements, especially giving priority to vehicles on the western approach.

Figure 2-12 Option for Prince Street / Menangle Street

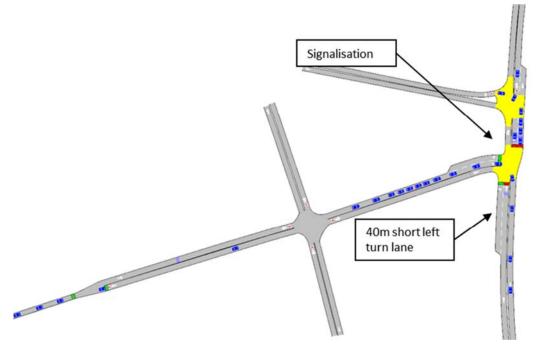


Table 2-3 presents the intersection performance results. The results indicate the proposed option significantly reduces the performance of the intersection from a LOS F to a LOS B in the AM peak, and LOS F to a LOS B in the PM peak when compared to the base case in 2026.

 Table 2-3
 Intersection Performance Prince Street / Menangle Street

	AM	РМ
Future Year 2026 Base	LOS F	LOS F
Future Year 2026 Option 1	LOS B	LOS B



2.2.3.3.2 Option 2

Option 2 proposed for this intersection is the same as Option 1 except the west approach (Prince Street) is one lane only (a shared left and right turn). The intersection performance results, as reported in Table 2-4, indicate there is no significant impact to the performance of the intersection by reducing the west approach to one lane. This is likely to be because there are relatively few left turners from Prince Street, with the vast majority of vehicles turning right in both peak hours.

Table 2-4 Inters	ection Performance	e Prince Stre	eet / Menangle Street

	AM	РМ
Future Year 2026 Option 1	LOS B	LOS B
Future Year 2026 Option 2	LOS B	LOS B

2.2.3.3.3 SIDRA Assessment

Option 1 and Option 2 were modelled in SIDRA v7.0 to optimise the signal control plan and validate the results produced by the Aimsun model. The SIDRA layouts of Option 1 and Option 2 are presented in Figure 2-13, the signal control plan in Figure 2-14 and the intersection results in Table 2-5. The SIDRA results indicate Option 1 and Option 2 have similar intersection performance, which is consistent with the results from Aimsun. For detailed movement summaries please see Appendix E.

Due to the reduced land acquisition, Option 2 is the recommended option. It is noted, however, that the single lane approach on Prince Street is a departure from RMS' preferred TCS layout. Early consultation with RMS should be undertaken to explain the reasons for the preferred design and seek approval in principle prior to proceeding with detailed design.

Figure 2-13 Prince Street / Menangle Street Option 1 and Option 2 SIDRA Layout

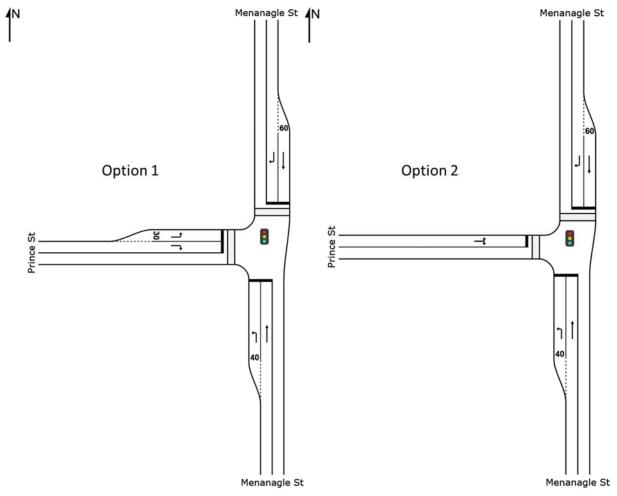




Figure 2-14 Prince Street / Menangle Street Proposed Signal Control Plan

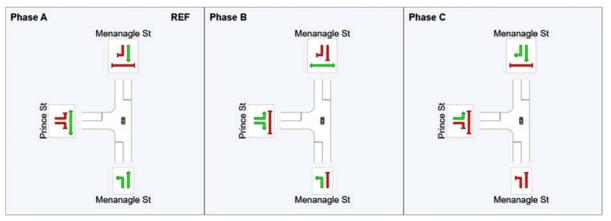


Table 2-5	SIDRA Intersection Performance Prince Street / Menangle Street			
		AM	РМ	
Future Yea	ar 2026 Option 1	LOS B	LOS C	
Future Yea	ar 2026 Option 2	LOS B	LOS C	

2.2.3.4 Prince Street / Argyle Street

The modelling for this intersection included the cumulative upgrades proposed for the other intersections within the study area and discussed above (Menangle Street / Argyle Street and Prince Street / Menangle Street).

The proposed option for this intersection is provision for a continuous left turn out of the eastern approach (Prince Street), formalising a 90m right turn bay on the southern approach (Argyle Street) and a reduction to one lane southbound on the northern approach (Argyle Street) as pictured below in Figure 2-15. This improves the intersection performance by removing conflicts for the left turn out of Prince Street, which is the movement that incurs the highest delay at the intersection.

Table 2-6 presents the intersection performance results. The results indicate the proposed option significantly reduces the average delay of the intersection from LOS E to LOS B in the PM peak when compared to the base case in 2026.



Figure 2-15 Option for Prince Street / Argyle Street

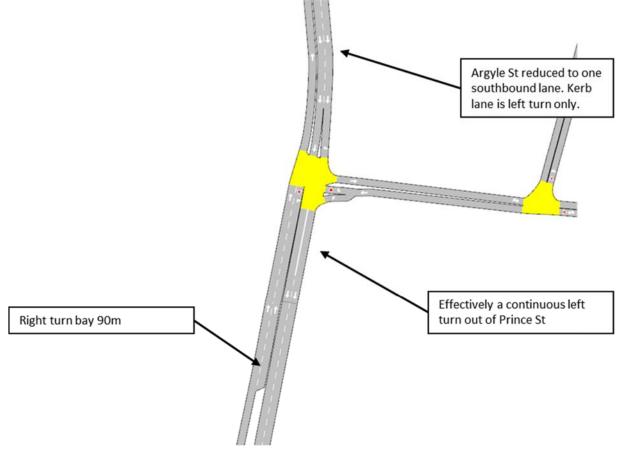


Table 2-6 Intersection Performance Prince Street / Argyle Street

	AM	РМ
Future Year 2026 Base	LOS B	LOS E
Future Year 2026 Option	LOS B	LOS B



2.2.3.5 Argyle Street / Barkers Lodge Road

The modelling for this intersection included the cumulative upgrades proposed for the other intersections within the study area and discussed above (Menangle Street / Argyle Street, Prince Street / Menangle Street and Prince Street / Argyle Street).

No further upgrades have been proposed for the Argyle Street / Barkers Lodge Road intersection because the cumulative impact of the options proposed thus far significantly improve the performance of the intersection, from a LOS F to a LOS E in the AM peak and LOS F to a LOS B in the PM peak. The intersection performance results are presented in Table 2-7.

Table 2-7	Intersection Performance Argyle Street / Barkers Lodge F	Road
-----------	--	------

	АМ	РМ
Future Year 2026 Base	LOS F	LOS F
Future Year 2026 "Cumulative Options"	LOSE	LOS B

The intersection improvement is due to a reduction in vehicles on Argyle Street northbound, increasing the number of gaps for vehicles to turn out of Barkers Lodge Road. The reduction of vehicles on Argyle Street northbound can be traced back to the improved performance of Prince Street and it's intersections with Argyle Street and Menangle Street, with more vehicles turning right into Prince Street from Argyle Street than was the case in the Future Year 2026 Base model. This analysis is summarised in Table 2-8 and pictured in Figure 2-16.



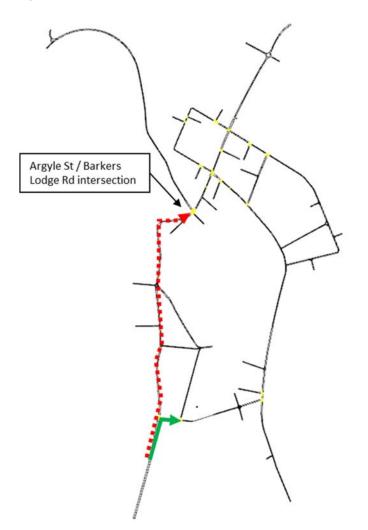




Table 2-8 Intersection Analysis of Argyle Street / Prince Street

	AM	AM (vph)		(vph)
	NB Through	NB Right Turn	NB Through	NB Right Turn
Future Year 2026 Base	1020	348	803	221
Future Year 2026 "Cumulative Options"	808	561	639	388
Difference	-212	+213	-164	+167

2.2.3.6 Argyle Street / Lumsdaine Street

The modelling for this intersection included the cumulative upgrades proposed for the other intersections within the study area and discussed above (Menangle Street / Argyle Street, Prince Street / Menangle Street and Prince Street / Argyle Street). This intersection is proposed to be upgraded to improve pedestrian safety and connectivity while minimising impact to nearby intersections.

2.2.3.6.1 Option 1

The proposed option for this intersection is traffic lights as illustrated in Figure 2-17. This option includes pedestrian crossings on three legs of the intersection. As a conservative assessment, 100 pedestrians per hour were modelled on each leg, replicating worst case conditions during school drop off and pick up periods.

Figure 2-17 Argyle Street / Lumsdaine Street – Option 1

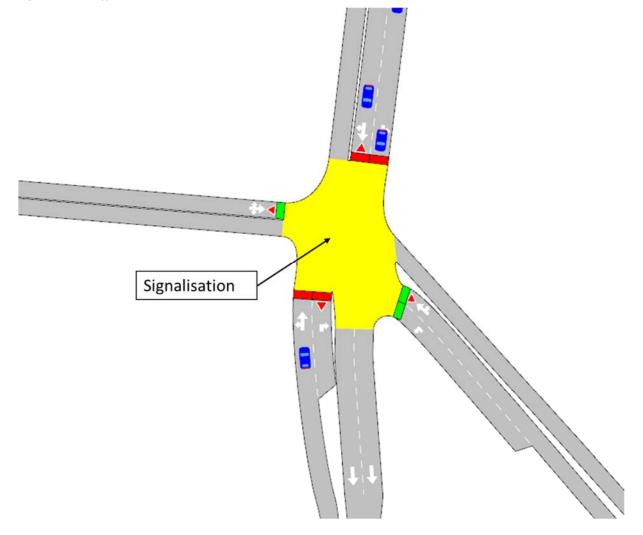




Table 2-9 presents the intersection performance results. The results indicate the proposed option maintains a good level of intersection performance with a LOS A in the AM and PM peaks when compared to the base case in 2026.

Table 2-9	Intersection Performa	on Performance Argyle Street / Lumsdaine Street (Option 1)		
		AM	РМ	
Future Yea	ar 2026 Base	LOS A	LOS A	
Future Yea	ar 2026 Option 1	LOS A	LOS A	

Given this intersection's proximity to the Argyle Street / Barkers Lodge Road intersection, the impact on the northern intersection performance was also assessed.

Table 2-10 presents the intersection performance results. The implementation of Argyle Street / Lumsdaine Street Option 1 results in a minor improvement in the AM peak intersection performance at Argyle Street / Barkers Lodge Road, evidenced by LOS D compared to LOS E without Option 1. This is most likely due to the signalised pedestrian crossing on Argyle Street creating additional gaps in downstream traffic.

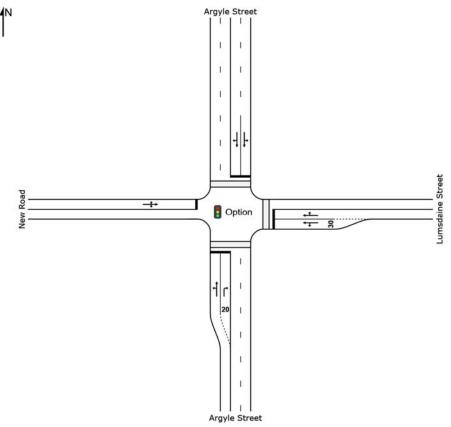
 Table 2-10
 Intersection Performance Argyle Street / Barkers Lodge Road (Option 1)

	AM	РМ
Future Year 2026 "Cumulative Options"	LOS E	LOS B
Future Year 2026 "Cumulative Options" plus Option 1	LOS D	LOS B

2.2.3.6.2 SIDRA Assessment

Option 1 was modelled in SIDRA v7.0 to optimise the signal control plan and validate the results produced by the Aimsun model. The SIDRA layout of Option 1 is presented in Figure 2-18, the signal control plan in Figure 2-19 and the intersection results in Table 2-11. The SIDRA results indicate Option 1 has good intersection performance with a LOS A in the AM and PM peaks, which is consistent with the results from Aimsun. For detailed movement summaries please see Appendix E.

Figure 2-18 Argyle Street / Lumsdaine Street Option 1 SIDRA Layout





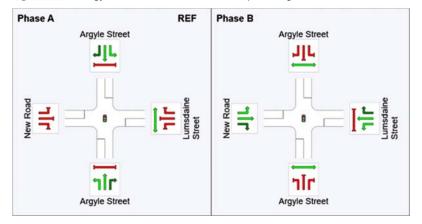


Figure 2-19 Argyle Street / Lumsdaine Street Proposed Signal Control Plan

Table 2-11 Argyle Street / Lumsdaine Street – Intersection Performance

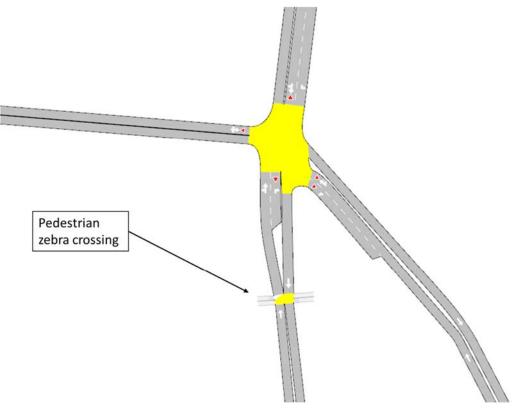
	АМ	РМ
Future Year 2026 Option 1 – Aimsun	LOS A	LOS A
Future Year 2026 Option 1 – SIDRA	LOS A	LOS A

2.2.3.6.3 Option 2

Option 2 proposes a pedestrian (zebra) crossing on Argyle Street just south of the Argyle Street / Lumsdaine Street intersection, as illustrated in Figure 2-20. To comply with Austroads requirements and reduce the overall speed environment, Argyle Street is proposed to be reduced to one lane southbound between Lumsdaine Street and Prince Street. This is consistent with the proposed option for Argyle Street / Prince Street intersection, which reduces Argyle Street to one lane southbound to facilitate the free flow left turn out of Prince Street.



Figure 2-20 Argyle Street / Lumsdaine Street – Option 2



It was assumed 100 pedestrians per hour would use this zebra crossing during peak hours. The impact on both Argyle Street / Lumsdaine Street and Argyle Street / Barkers Lodge Road intersections were assessed.

Table 2-12 presents the Argyle Street / Lumsdaine Street intersection performance results. The results indicate the proposed option maintains a good level of intersection performance with a LOS B in the AM and PM peaks.

Table 2-12 II	ntersection Performance	Argyle Street	Lumsdaine Street	(Option 2)
---------------	-------------------------	---------------	------------------	------------

	AM	РМ
Future Year 2026 Base	LOS A	LOS A
Future Year 2026 Option 2	LOS B	LOS B

Table 2-13 presents the Argyle Street / Barkers Lodge Road intersection performance results. The results show Option 2 gives a minor improvement to the intersection performance at Argyle Street / Barkers Lodge Road, with a LOS D compared to a LOS E without Option 2 during the AM peak.

Table 2-13 Intersection Performance Argyle Street / Barkers Lodge Road (Option 2)

	AM	РМ
Future Year 2026 "Cumulative Options"	LOS E	LOS B
Future Year 2026 "Cumulative Options" plus Option 2	LOS D	LOS B

The modelling results discussed above show that the proposed pedestrian zebra crossing and reduction of capacity on Argyle Street southbound between Lumsdaine Street and Prince Street are not anticipated to have a significant negative impact on the road network in 2026.

It can be seen that both Option 1 and Option 2 result in improved pedestrian amenity and minor traffic performance benefits. The preferred option is subject to further design development and discussion with relevant stakeholders, in particular Roads and Maritime and Picton Public School.

2.3 Traffic modelling summary

Cardno estimated the future year 2026 road network conditions and established a future baseline scenario for comparative assessment of options, by developing Aimsun microsimulation traffic models for the study area. The traffic models indicated the impact of future development and traffic growth on the road network resulted in capacity issues at the following intersections by 2026:

- > Menangle Street / Argyle Street
- > Prince Street / Menangle Street
- > Prince Street / Argyle Street
- > Argyle Street / Barkers Lodge Road.

Preferred options were proposed to address the identified short term capacity issues, and their impact was comparatively assessed against the future baseline scenario. The proposed options are:

Intersection	Proposed Option
Menangle Street / Argyle Street	 Right turn ban on the eastern approach (Menangle Street)
Argyle Street / Margaret Street	 Increase maximum green time of the signal control plan on the eastern approach from 15s to 20s
Prince Street / Menangle Street	 Upgrade the intersection to signals
Prince Street / Argyle Street	 Reduce northern approach (Argyle Street) to one lane southbound Provide for a continuous left turn out of Prince Street Formalise the 90m right turn bay on the southern approach (Argyle Street)
Argyle Street / Barkers Lodge Road	No further upgrades required assuming the above options are in place

The proposed options result in improved intersection performance as summarised below :



Intersection	A	M	РМ			
	2026 Base	Option	2026 Base Opti			
Menangle Street / Argyle Street	LOS D	LOS C	LOS F	LOS D		
Argyle Street / Margaret Street	LOS D	LOS D	LOS D	LOS D		
Prince Street / Menangle Street *	LOS F	LOS B	LOS F	LOS B		
Prince Street / Argyle Street	LOS B	LOS B	LOS E	LOS B		
Argyle Street / Barkers Lodge Road **	LOS F	LOS E	LOS F	LOS B		

* Option 2 for this intersection is preferred and recommended

** No upgrades are proposed for this intersection, "option" considers the cumulative impact of all the other proposed options, not including Argyle St / Lumsdaine St

Additionally, two options to improve pedestrian safety and connectivity at the Argyle Street / Lumsdaine Street intersection were proposed and assessed. Both options improve pedestrian safety and show minor improvement to the Argyle Street / Barkers Lodge Road intersection performance.

Aunula Cérect / Lumadaina Cérect Internaction	AM		PM			
Argyle Street / Lumsdaine Street Intersection	2026 Base	Option	2026 Base	Option		
Option 1 – install traffic lights	LOS A	LOS A	LOS A	LOS A		
Option 2 – install pedestrian (zebra crossing) on the southern leg of the intersection	LOS A	LOS B	LOS A	LOS B		

The proposed options are recommended because they address the short term capacity issues which are anticipated in the road network by 2026 and demonstrate cumulative network wide benefits for relatively low cost.

3 Strategic Design Development

3.1 Design Guides and References

The strategic intersection designs of Menangle Street / Prince Street, and Argyle Street / Prince Street have been based of the following guides and references:

- > Wollondilly Shire Council Design Specifications Subdivision and Engineering Standards
- > Wollondilly Shire Council Standard Drawings Subdivision and Engineering Standards
- > Austroads Part 3 Geometric Design
- > Austroads Part 4A Unsignalised and Signalised Intersections
- > Roads and Maritime Services Traffic Signal Design Appendix D
- > Roads and Maritime Services Delineation Guide Section 4 Longitudinal Markings
- > Roads and Maritime Services Delineation Guide Section 4 Transverse Markings
- > Roads and Maritime Services Delineation Guide Section 4 Pavement Arrows
- > Rawlinson's Australian Construction Handbook Edition 26 (2016)

3.2 Menangle Street / Prince Street

The design of the intersection layout for the Menangle Street and Prince Street is a signalised urban channelised T-junction, with a total of eight (8) new traffic lanterns. The intersection has been designed for a 5.2 m passenger vehicle, with an 8.8 m service vehicle as the check vehicle. These vehicles have been chosen for the design as the Prince Street Bridge enforces a load limit of 5 t gross and access to other streets from Prince Street are residential dwellings only.

The intersection consists of a channelised right turn movement for southbound traffic into Prince Street and a channelised left turn movement for northbound traffic into Prince Street. The channelising of these turn movements are implemented through the use of both chevron marked medians and a concrete median. The lane length of the right turn movement has been dictated by Austroad guidelines, whereas the left turn movement lane length has been determined through the Aimsun and SIDRA modelling outputs.

The kerb returns on Prince Street have been improved to allow for the design and check vehicles to manoeuvre safely through the turn movements which has resulted in pavement widening and installation of new kerb and gutter. Further pavement widening and new kerb and gutter has occurred on the eastern side of Argyle Street to allow for a 3.5 m through lane for the southbound traffic.

The traffic signal phasing for the proposed intersection upgrade has been matched to the SIDRA modelling inputs as per Section 2.2.3.3.

3.2.1 Cost Estimation

A cost estimate for the construction of the proposed works at the intersection of Menangle Street / Prince Street was performed using material and volume take-offs from strategic design depicted on drawing 82018177-01 SK006 (refer to Appendix A). Cardno also utilised current industry costs for the relevant elements of the works based on Cardno's previous experience on similar projects, as well as Rawlinson's cost guide handbook.

The estimate includes the following costs:

- > Pre-construction or site preparation works
- > Construction of all elements as shown in the drawings, unless specifically excluded
- > Contingency allowance appropriate to the stage of the estimate

A summary of the cost estimate can be found in Table 3-1 below. For the detailed cost estimate please refer to Appendix B.

Table 3-1 Menangle Street / Prince Street Cost Estimate Summary

General Items	\$42,500.00
Earthworks	\$7,967.00
Roadworks	\$358,600.00
Erosion & Sedimentation Control	\$5,000.00
Total Excl. Contingency	\$414,067.00
Contingency 30%	\$124,220.10
Total Incl. Contingency	\$538,287.10

It should be noted no allowance has been made for any utility relocation or protection works as there is insufficient information to determine works at this stage. It should also be noted the cost estimate above has assumed the works for Menangle Street / Prince Street are constructed in isolation. If Council were to utilise a contractor to construct additional works at the same time, there would likely be cost savings in preliminaries and site establishment.

3.3 Argyle Street / Prince Street

The design of the intersection layout for the Argyle Street / Prince Street is an unsignalised urban channelised T-junction. The intersection has been designed for a 5.2 m passenger vehicle, with an 8.8 m service vehicle as the check vehicle. These vehicles have been chosen for the design as the Prince Street Bridge enforces a load limit of 5 t gross and access to other streets from Prince Street are residential dwellings only.

The intersection consists of a channelised right turn movement for northbound traffic into Prince Street, a dedicated left turn movement for southbound traffic into Prince Street and a channelised left turn movement for westbound traffic into Argyle Street. The channelising of these turn movements are implemented through the use of chevron marked medians. The lane length of the left turn movements into Argyle Street has been dictated by Austroad guidelines, whereas the right turn movement lane length has been determined through Aimsun modelling outputs.

Pavement widening is to be implemented for the channelised left turn movement into Argyle St and for the northbound through lane on Argyle Street. This shall also result in new kerb and gutter for these sections of pavement.

3.3.1 Cost Estimation

A cost estimate for the construction of the proposed works at the intersection of Argyle Street and Prince Street was performed using material and volume take-offs from the strategic design depicted on drawing 82018177-01 SK009 (refer to Appendix A). Cardno also utilised current industry costs for the relevant elements of the works based on Cardno's previous experience on similar projects, as well as Rawlinson's cost guide handbook.

The estimate includes the following costs:

- > Pre-construction or site preparation works
- > Construction of all elements as shown in the drawings
- > Contingency allowance appropriate to the stage of the estimate

A summary of the cost estimate can be found in Table 3-2 below. For the detailed cost estimate please refer to Appendix B.



Table 3-2 Argyle Street / Prince Street Cost Estimate Summary

General Items	\$36,000.00
Earthworks	\$74,530.00
Roadworks	\$82,764.45
Erosion & Sedimentation Control	\$5,000.00
Total Excl. Contingency	\$198,294.45
Contingency 30%	\$59,488.33
Total Incl. Contingency	\$257,782.78

It should be noted no allowance has been made for any utility relocation or protection works as there is insufficient information to determine works at this stage. It should also be noted the cost estimate above has assumed the works for Argyle Street / Prince Street are constructed in isolation. If Council were to utilise a contractor to construct additional works at the same time, there would likely be cost savings in preliminaries and site establishment.

4 Conclusions

4.1 Overview

The Picton Town Centre Transport Master Plan (TDG, 2017) proposed numerous road upgrades, including the replacement of the Prince Street bridge, to improve traffic conditions in the town centre. The TDG proposals will require long term planning and funding arrangements in order to be implemented in full, due to the large extent of works and private property acquisition required. It is also likely the Picton Town Centre Transport Master Plan will be revised due to development re-zoning in the Greater Macarthur region.

In this revised 'Picton Town Centre Transport Plan 2026', Cardno has identified a number of short term network capacity improvements as interim measures and developed designs sufficient for strategic cost estimates and consultation with relevant stakeholders.

4.2 Proposed Intersection Improvements

The upgrades developed as part of this transport plan are short term, "quick win" measures can be implemented at relatively low cost and provide sufficient network capacity until at least 2026. Table 4-1 summarises the proposed upgrades.

Intersection	Proposed Option
Menangle Street / Argyle Street	 Right turn ban on the eastern approach (Menangle Street)
Argyle Street / Margaret Street / Cliffe Street	 Increase maximum green time of the signal control plan on the eastern approach from 15s to 20s
Menangle Street / Prince Street	Signalise the intersection
Argyle Street / Prince Street	 Reduce the northern approach (Argyle Street) to one lane southbound Provide for a continuous left turn out of Prince Street Formalise the 90m right turn bay on the southern approach (Argyle Street)
Argyle Street / Barkers Lodge Road	 No upgrades required assuming the above options are implemented
Argyle Street / Lumsdaine Street	 Option 1 - signalise the intersection Option 2 - install pedestrian (zebra) crossing on Argyle Street south of the intersection

Table 4-1 Summary of proposed intersection improvements

4.3 Strategic Cost Estimates

Estimated costs for the main capital works are shown in Table 4-2.

Table 4-2 Strategic cost estimates for proposed intersection upgrades

Intersection	Strategic Cost Estimate (incl. contingency @ 30%)
Menangle St / Prince Street	\$538,287.10
Argyle Street / Prince Street	\$257,782.78

APPENDIX



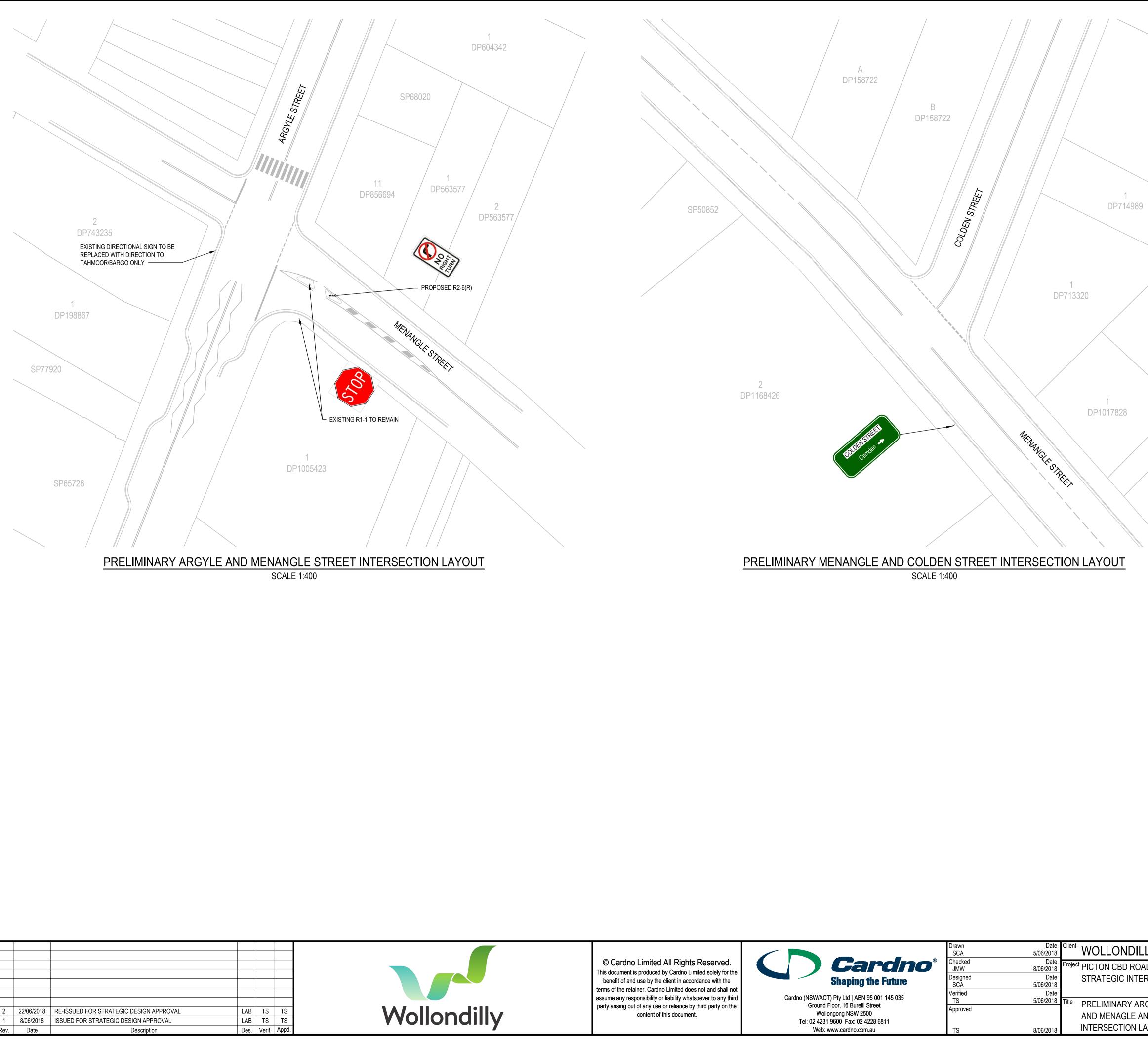
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<u>NOTES</u>

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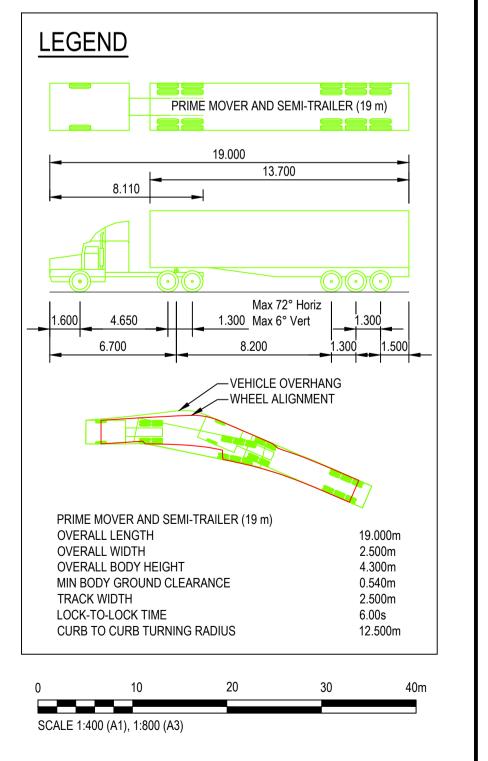
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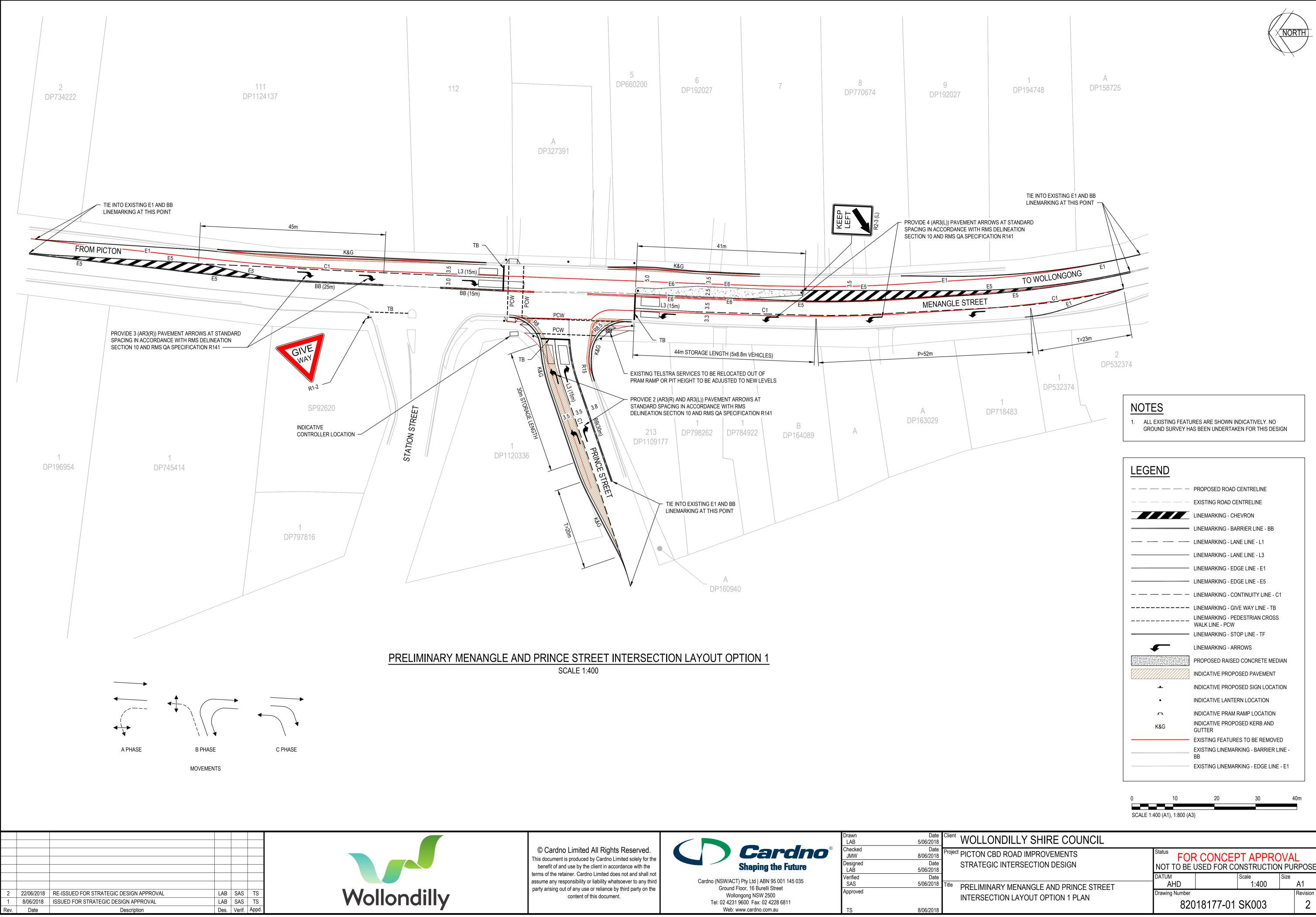
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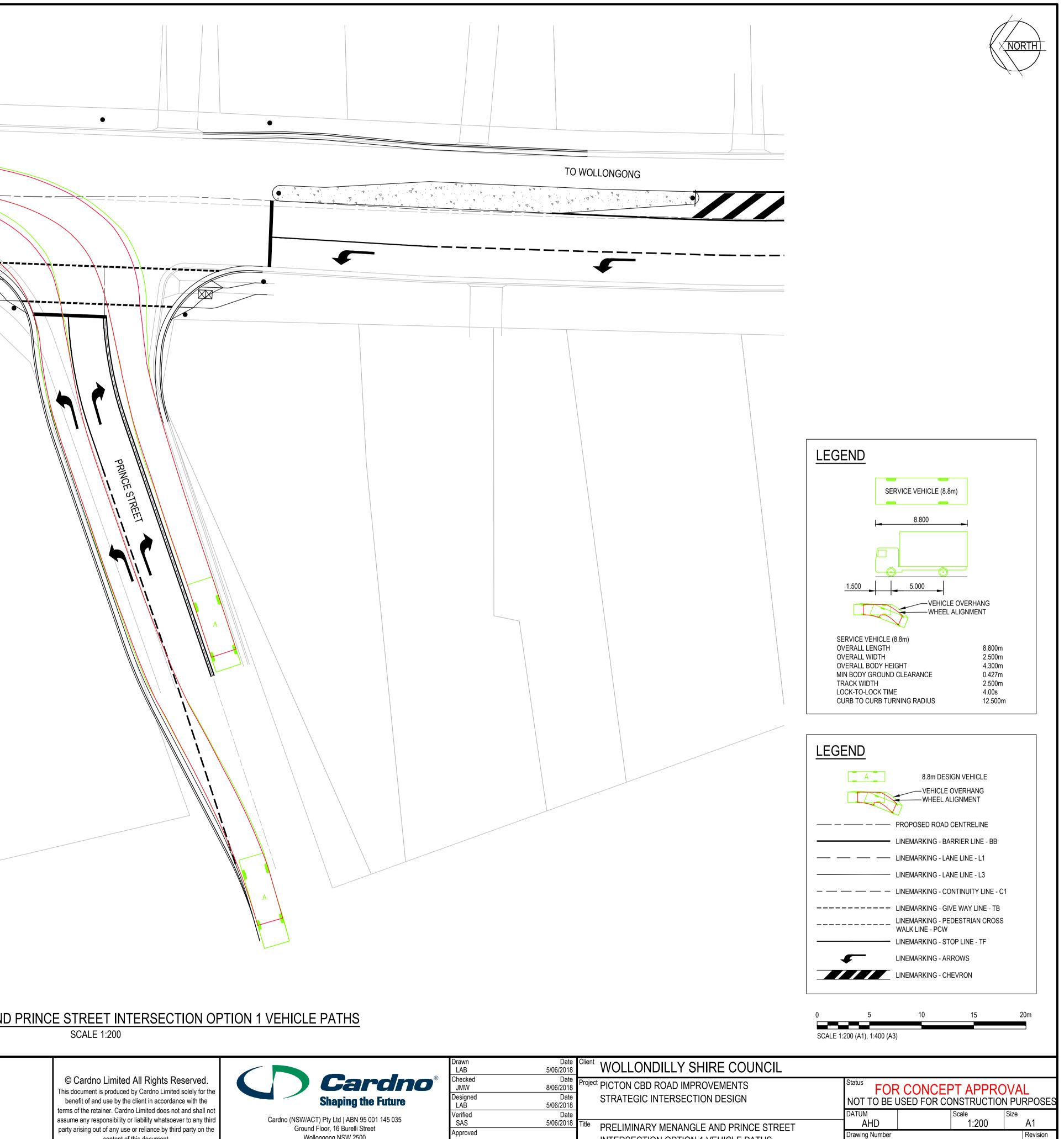




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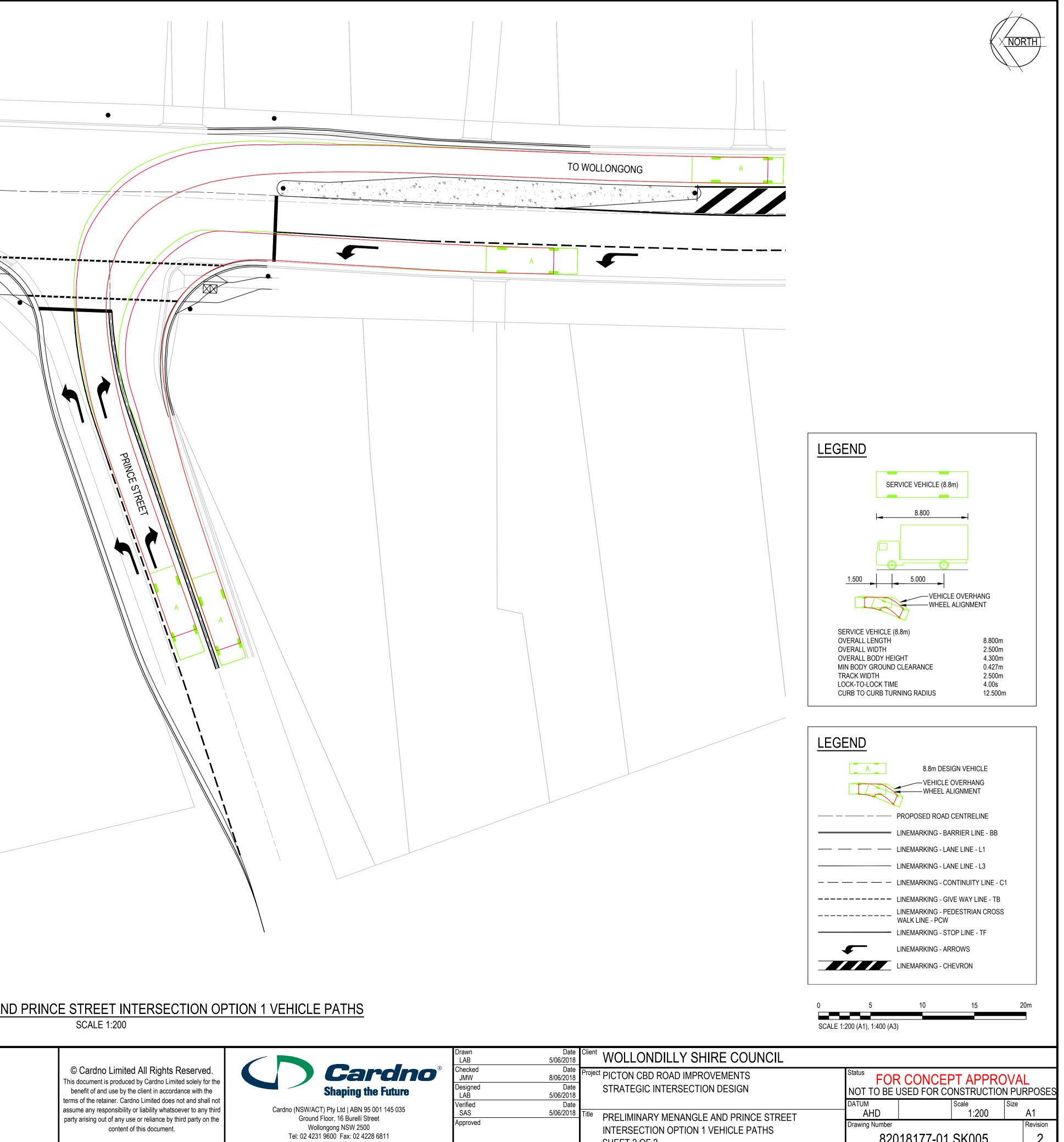
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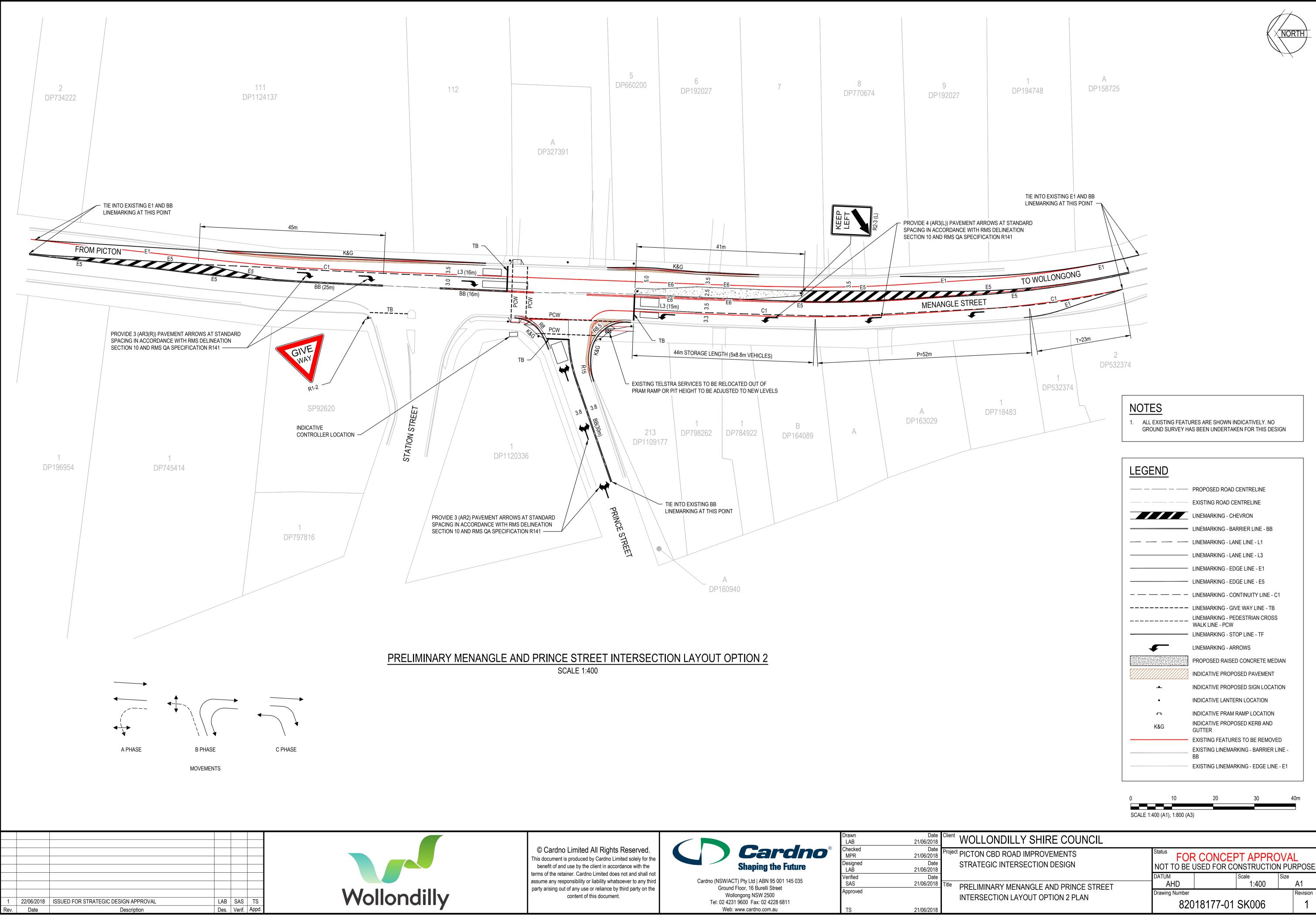


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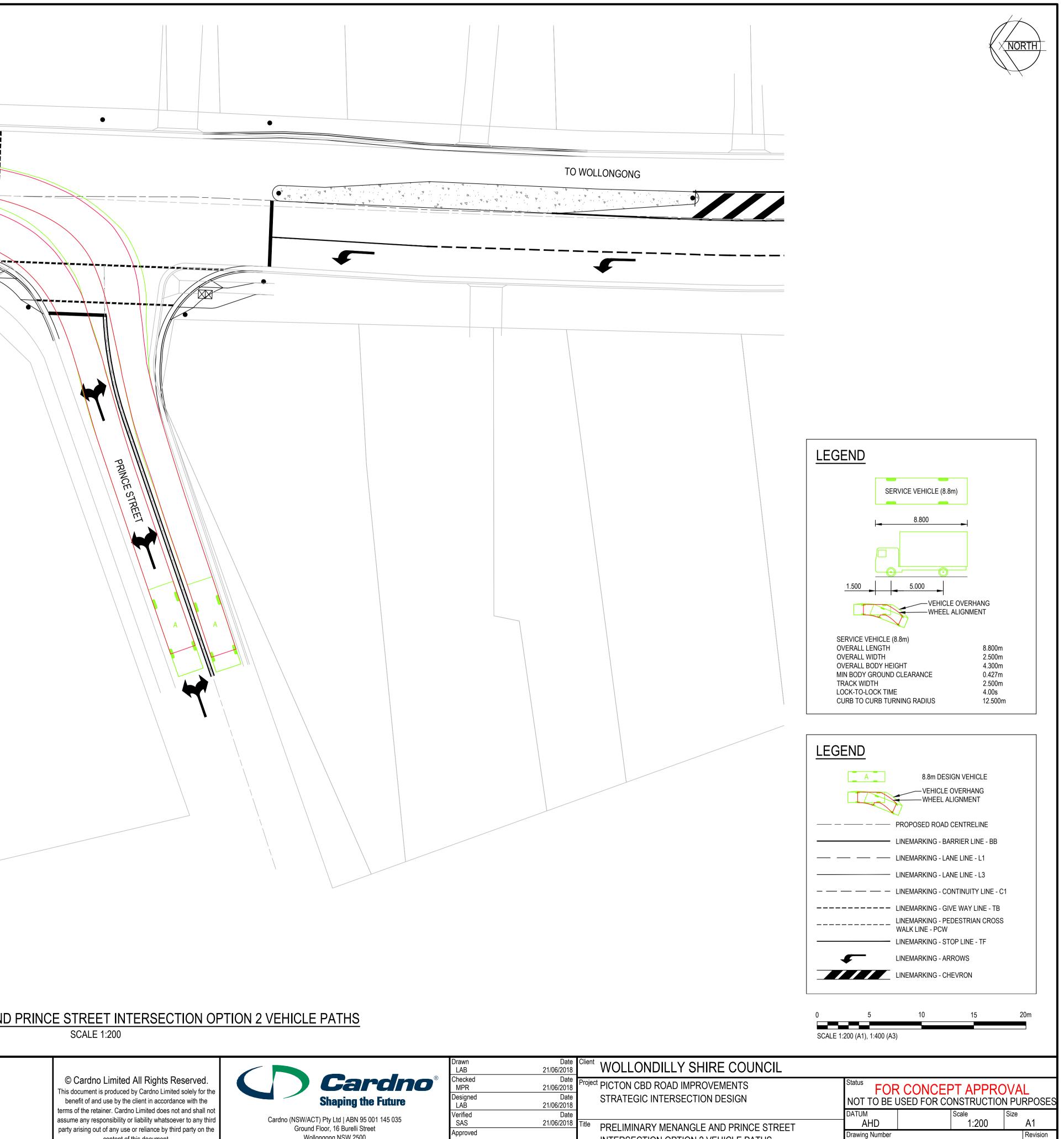


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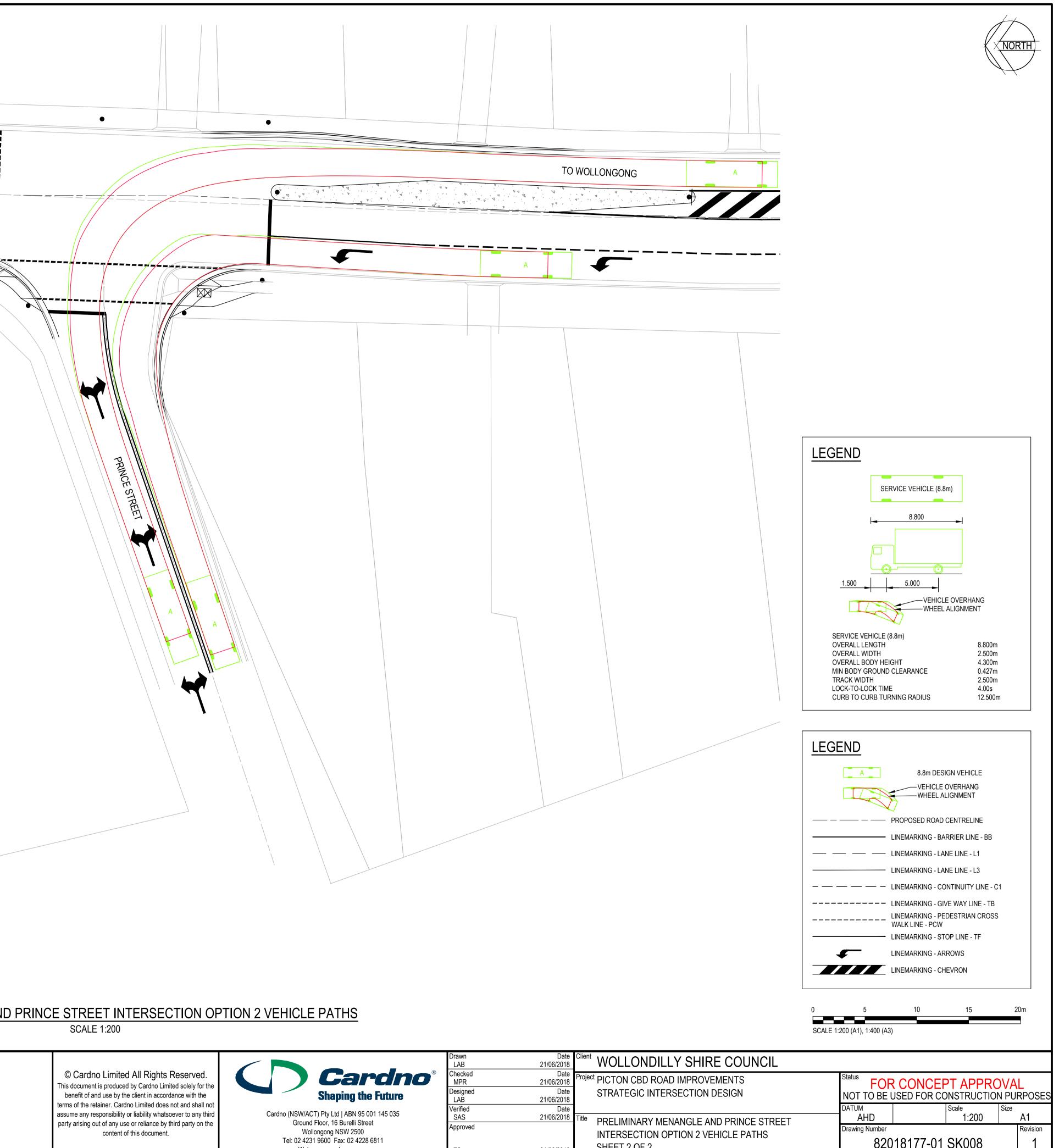
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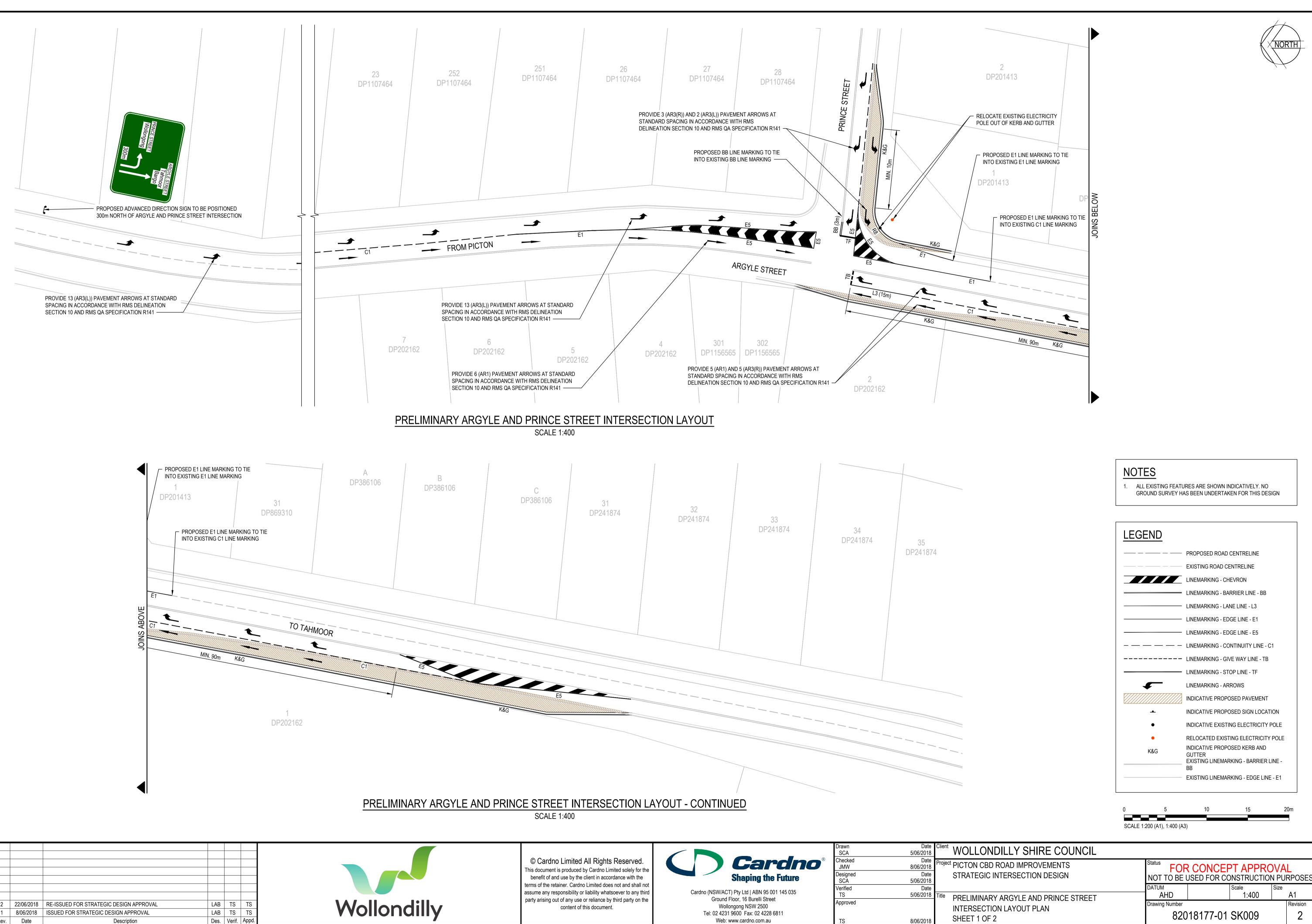
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82018177-01 SK008

1



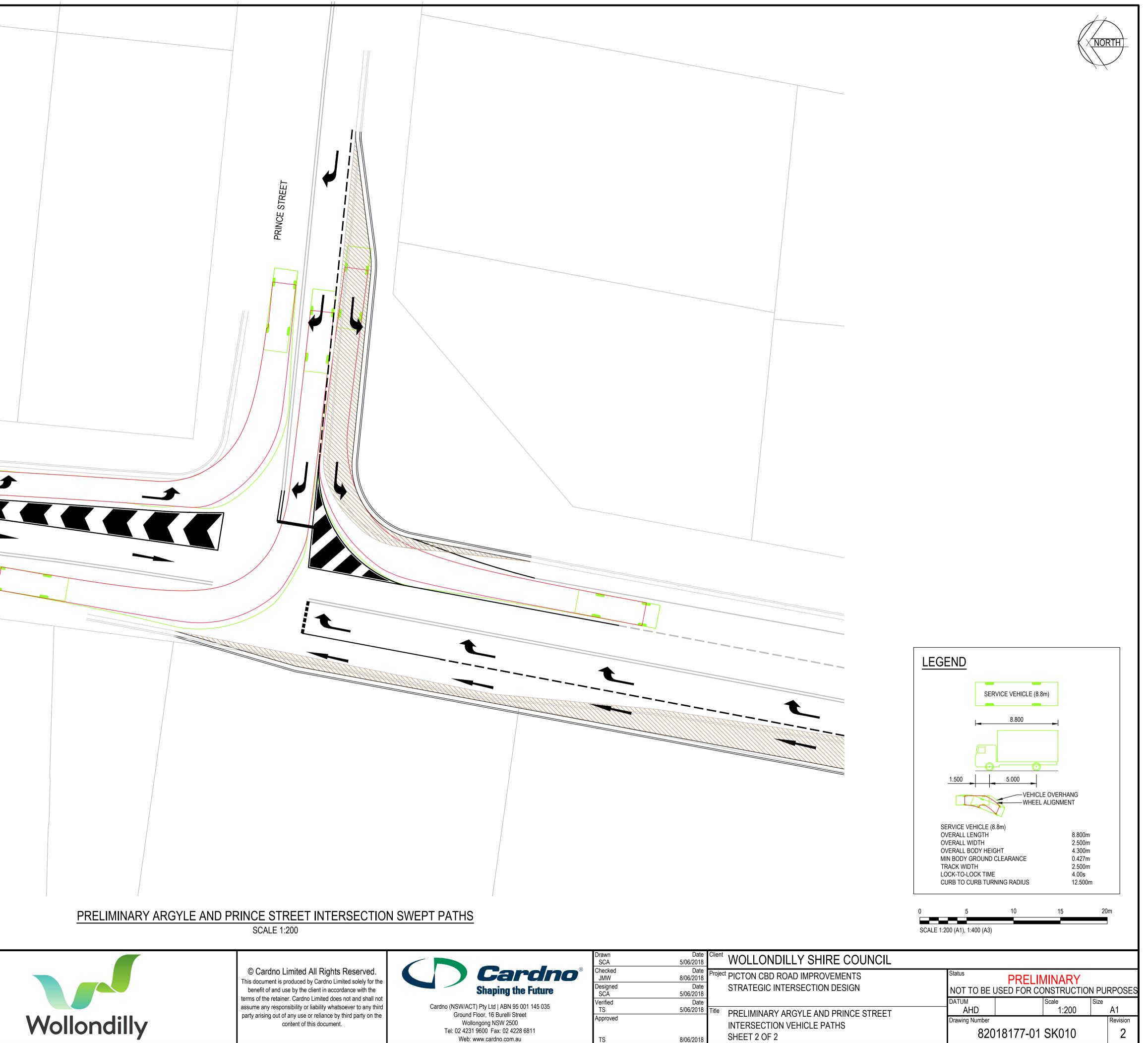


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						PRELIMINARY
2	22/06/2018 8/06/2018	RE-ISSUED FOR STRATEGIC DESIGN A ISSUED FOR STRATEGIC DESIGN APPI Descriptio	ROVAL	LABTSLABTSLABTSTSTSDes.Verif.Appd.	W	ollondilly





SCA	5/06/2018	
Checked JMW	Date 8/06/2018	Project PICTON CBD ROA
Designed SCA	Date 5/06/2018	STRATEGIC INTER
Verified TS	Date 5/06/2018	Title PRELIMINARY AR
Approved		INTERSECTION VE
TS	8/06/2018	SHEET 2 OF 2

APPENDIX



STRATEGIC COST ESTIMATES



PICTON CBD INTERSECTION DESIGN Revision 0 Cardno INTERSECTION OF ARGYLE AND PRINCE STREET Date 21/06/2018 GENERAL ITEMS 1.0 Details Quantity Unit Item Rate Total 1.1 Preliminaries ite establishment including site office, site facilities, signage, compliance with OH&S Act and 1.1.1 Regulations, construction and maintenance of site access tracks, site supervision and removal on 1 item \$5.000.00 \$5.000.00 ompletion. rovision for traffic including preparation and implementation of traffic management plan for all te access, including all application fees for Council S138, drawings, revisions, Council liaison as \$16,000.00 \$16,000.00 1.1.2 1 item equired, installation & maintenance of all traffic management devices for road closures, shoulder losures, detours etc. for the duration of construction. Survey - including services locations, services connection points, set out works and the provision of engineering survey control for all aspects of the Works (incl WAE survey for utilities) and review 1.1.3 1 item \$5,000.00 \$5,000.00 of DBYD plans Prepare and implement site management plan, environmental management plan WHS 1.1.4 1 item \$2,000.00 \$2,000.00 nanagement plan and quality plan. Supply, install & maintenance of ATF fencing for any areas the Contractor deems required. 1.1.5 200 m \$30.00 \$6,000.00 Maintain fencing for the duration of the civil contract and remove upon construction completion. 1.2 **Clearing & Site Preparation** Clearing of site, remove & dispose of all rubbish, fences and debris as instructed by the 1 \$2,000.00 1.2.1 \$2,000.00 item superintendent, & dispose off-site (including tip fees). Sub Total for General Items \$36,000.00 BULK EARTHWORKS 2.0 Details Quantity Unit Rate Item Total 2.1 Bulk Earthworks and Materials Management Cut to Stockpile for new pavement area (all classes of material, solid measurement, no allowance 2.1.1 290 cu.m \$7.00 \$2,030,00 or bulking factors) \$72,500.00 2.1.2 Cart and stockpile excess fill to nearest waste management facility 290 cu.m Sub Total for Bulk Earthworks \$74.530.00 3.0 ROADWORKS Item Details Quantity Unit Rate Total 3.1 Road Pavement for Collector Road 3.1.1 Trim and compact subgrade material to subgrade design level 522 \$2.50 \$1,304.90 sq.m Supply, place, compact and trim sub-base material DGS40, 300mm minimum consolidated \$18,268.57 3.1.2 522 sa.m \$35.00 acted in accordance with Council's Construction Specification Supply, place, compact and trim base material DGB20, 200mm minimum consolidated thickness compacted in accordance with Council's Construction Specification 3.1.3 522 sq.m \$25.00 \$13 048 98 Supply and place AC14 in 50mm compacted thickness including primer seal \$22,500.00 3.1.4 750 sq.m \$30.00 3.2 Kerbina onstruct Council's standard kerb & gutter including transition to pits & other kerb types, joints & 3.2.1 221 \$55.00 \$12,133.83 m drainage outlets 3.3 Road Signs 3.3.1 Supply and install 'advanced turn' sign only (R1-2) as per AS1742.2 \$500.00 \$500.00 1 ea. 3.4 Road Delineation 3.4.1 Install 'BB' type line marking as per RMS QA Specification R141 3.4 m \$5.00 \$16.75 3.4.2 nstall 'E1' type line marking as per RMS QA Specification R141 41.8 \$4.00 \$167.20 nstall 'E5' type line marking as per RMS QA Specification R141 \$770.00 3.4.3 192.5 m \$4.00 nstall 'L3' type line marking as per RMS QA Specification R141 3.4.4 15.0 \$2.00 \$30.00 m Install 'C1' type line marking as per RMS QA Specification R141 3.4.5 \$1,037.75 230.6 m \$4.50 3.4.6 nstall 'TB' type line marking as per RMS QA Specification R141 4.0 m \$15.00 \$60.00 Install 'TF' type line marking as per RMS QA Specification R141 \$15.00 \$59.49 3.4.7 4.0 m 3.4.8 Install Chevron markings as per RMS QA Specification R141 96.2 \$35.00 \$3,367.00 sq.m

3.4.9	Install "AR3(R)" type pavement arrow as per RMS QA Specification R141	11.0	ea.	\$100.00	\$1,100.00
3.4.10	Install "AR3(L)" type pavement arrow as per RMS QA Specification R141	12.0	ea.	\$100.00	\$1,200.00
0.4.10		12.0	64.	\$100.00	ψ1,200.00
3.4.11	Install "ARR5" type pavement arrow as per RMS QA Specification R141	11.0	ea.	\$100.00	\$1,100.00
3.4.12	Install "AR1" type pavement arrow as per RMS QA Specification R141	11.0	ea.	\$100.00	\$1,100.00
3.5	Geotechnical Assessment				
3.5.1	Geotechnical investigation and testing of road pavements onsite	1	item	\$5,000.00	\$5,000.00
	Sub Total for Roadworks				\$82,764.45
4.0	EROSION & SEDIMENTATION CONTROL				
Item	Details	Quantity	Unit		Total
4.1	Soil & Water Management				
4.1.1	Erosion and sediment control measures	1	item	\$5,000.00	\$5,000.00
	Sub Total for Erosion and Sediment Control				\$5,000.00
	TOTAL EXCL. CONTINGEN	ICY			\$198,294.45
	Contingency				\$59.488.33
	Application of 30% Contingency				
	TOTAL INCL. CONTINGEN	CY			\$257,782.78
Notes					
	1 These preliminary quantities and budget estimates are an indicative preliminary engineering				
	estimate only, based upon a preliminary design and does not include detailed design, tendering,				
	construction management/superintendent or land acquisition fees. Rates are based on Cordell's and Rawlinson's and Cardno's engineering experience on similar civil projects and quantities are				
	estimated based on the preliminary level of design to date. Cardno is not a Quantity Surveyor and				
	these are not Quantity Surveyor estimates and as such should not be relied upon for final				
	budgeting purposes.				
	² Pavement has been assumed to incorporate 300mm layer DGS40, 200mm layer DGB20 and 50mm layer AC14				
	3 Asphaltic Concrete layer assumed to overlap existing pavement by 1m				
	4 No allowance has been made for any utility relocation or protection works as there is not enough				
	information to determine works at this stage				

5 Works assumed to be completed in a 2 week program

L

PICTON CBD INTERSECTION DESIGN Revision 0 Cardno INTERSECTION OF MENANGLE AND PRINCE STREET Date 21/06/2018 GENERAL ITEMS 1.0 Details Quantity Unit Rate Item Total 1.1 Preliminaries ite establishment including site office, site facilities, signage, compliance with OH&S Act and 1.1.1 Regulations, construction and maintenance of site access tracks, site supervision and removal on 1 item \$5.000.00 \$5.000.00 ompletion. rovision for traffic including preparation and implementation of traffic management plan for all te access, including all application fees for Council S138, drawings, revisions, Council liaison as \$24,000.00 \$24,000.00 1.1.2 1 item equired, installation & maintenance of all traffic management devices for road closures, shoulder losures, detours etc. for the duration of construction. Survey - including services locations, services connection points, set out works and the provision of engineering survey control for all aspects of the Works (incl WAE survey for utilities) and review 1.1.3 1 item \$5,000.00 \$5,000.00 of DBYD plans Prepare and implement site management plan, environmental management plan WHS 1.1.4 1 item \$2,000.00 \$2,000.00 nanagement plan and quality plan. Supply, install & maintenance of ATF fencing for any areas the Contractor deems required. 1.1.5 150 m \$30.00 \$4,500.00 Maintain fencing for the duration of the civil contract and remove upon construction completion. 1.2 **Clearing & Site Preparation** Clearing of site, remove & dispose of all rubbish, fences and debris as instructed by the 1 \$2,000.00 1.2.1 \$2,000.00 item superintendent, & dispose off-site (including tip fees). Sub Total for General Items \$42,500.00 BULK EARTHWORKS 2.0 Unit Details Quantity Rate Item Total 2.1 Bulk Earthworks and Materials Management Cut to Stockpile for new pavement area (all classes of material, solid measurement, no allowance 2.1.1 31 cu.m \$7.00 \$217.00 or bulking factors) \$7,750.00 2.1.2 Cart and stockpile excess fill to nearest waste management facility 31 cu.m Sub Total for Bulk Earthworks \$7.967.00 3.0 ROADWORKS Item Details Quantity Unit Rate Total Road Pavement for Collector Road 3.1 3.1.1 Trim and compact subgrade material to subgrade design level 60 sq.m \$2.50 \$150.00 Supply, place, compact and trim sub-base material DGS40, 300mm minimum consolidated 3.1.2 60 sa.m \$35.00 \$2,100.00 acted in accordance with Council's Construction Specification Supply, place, compact and trim base material DGB20, 200mm minimum consolidated thickness compacted in accordance with Council's Construction Specification 3.1.3 60 sq.m \$25.00 \$1,500.00 Supply and place AC14 in 50mm compacted thickness including primer seal \$6,000.00 3.1.4 200 sq.m \$30.00 3.2 Kerbina onstruct Council's standard kerb & gutter including transition to pits & other kerb types, joints & 3.2.1 140 m \$55.00 \$7,700.00 drainage outlets 3.2.2 Concrete median island 100mm with SL82 mesh \$70.00 \$5,950.00 85 sq.m \$2,100.00 3.2.3 Pedestrian pram ramp ea \$700.00 3.3 Road Signs 3.3.1 \$315.00 \$315.00 Supply and install 'give way' sign only (R1-2) as per AS1742.2 ea. 1 3.3.2 Supply and install 'keep left' sign (R2-3) as per AS1742.2 \$315.00 \$315.00 3.4 Road Delineation Install 'BB' type line marking as per RMS QA Specification R141 3.4.1 78 m \$5.00 \$390.00 3.4.2 nstall "PCW" type line markings as per RMS QA Specification R141 61 \$5.00 \$305.00 3.4.3 Install 'E1' type line marking as per RMS QA Specification R141 130 m \$4.00 \$520.00 3.4.4 nstall 'E5' type line marking as per RMS QA Specification R141 300 \$4.00 \$1,200.00 m Install 'E6' type line marking as per RMS QA Specification R141 \$360.00 3.4.5 90 m \$4.00

3.4.6	Install 'L3' type line marking as per RMS QA Specification R141	35	m	\$2.00	\$70.00
3.4.7	had 1004 has a financial france and DNO OA On a 1955 fran D444			\$4.50	8405.00
3.4.7	Install 'C1' type line marking as per RMS QA Specification R141	90	m	\$4.50	\$405.00
3.4.8	Install 'TB' type line marking as per RMS QA Specification R141	30	m	\$14.00	\$420.00
5.4.0				\$14.00	ψ 4 20.00
3.4.9	Install Chevron markings as per RMS QA Specification R141	80	sq.m	\$35.00	\$2,800.00
0.1.0	notal energe as per une ar operindation (11)		oq.m	¢00.00	φ2,000.00
3.4.10	Install "AR3(R)" type pavement arrow as per RMS QA Specification R141	3	ea.	\$100.00	\$300.00
3.4.11	Install "AR3(L)" type pavement arrow as per RMS QA Specification R141	4	ea.	\$100.00	\$400.00
3.4.12	Install "AR2" type pavement arrow as per RMS QA Specification R141	3	ea.	\$100.00	\$300.00
3.5	Traffic Signals				
3.5.1	Installation, and supply of Traffic lanterns as per relevant standards	8	ea.	\$40,000.00	\$320,000.00
	Or a familiar l American mant				
3.6	Geotechnical Assessment				
3.6.1	Geotechnical investigation and testing of road pavements onsite	1	item	\$5,000.00	\$5,000.00
5.0.1	Geolecificar investigation and testing of road pavements of site	1	item	\$3,000.00	45,000.00
	Sub Total for Roadworks				\$358,600.00
					\$555,555.55
4.0	EROSION & SEDIMENTATION CONTROL				
Item	Details	Quantity	Unit		Total
		•			
4.1	Soil & Water Management				
	-				
4.1.1	Erosion and sediment control measures	1	item	\$5,000.00	\$5,000.00
	Sub Total for Erosion and Sediment Control				\$5,000.00
	TOTAL EXCL. CONTINGEI	NCY			\$414,067.00
	Continuonou				6404 000 40
	Contingency				\$124,220.10
_	Application of 30% Contingency		_	_	¢E20 207 40
	TOTAL INCL. CONTINGEN				\$538,287.10

Notes:

- 55: These preliminary quantities and budget estimates are an indicative preliminary engineering estimate only, based upon a preliminary design and does not include detailed design, tendering, construction management/superintendent or land acquisition fees. Rates are based on Cordell's 1 and Rawlinson's and Cardno's engineering experience on similar civil projects and quantities are estimated based on the preliminary level of design to date. Cardno is not a Quantity Surveyor and these are not Quantity Surveyor estimates and as such should not be relied upon for final budgeting ourcoses.
 2 Pavement has been assumed to incorporate 300mm layer DGS40, 200mm layer DGB20 and 50mm layer AC14

- A Saphalic Concrete layer assumed to overlap existing pavement by 1m
 No allowance has been made for any utility relocation or protection works as there is not enough
 information to determine works at this stage
 5 Works assumed to be completed in a 3 week program

APPENDIX



RSA – EXISTING CONDITIONS



Road Safety Audit

Picton Town Centre – Various Locations – Existing Conditions

8201817701

Prepared for Cardno NSW/ACT Pty Ltd

May 2018







Contact Information

Document Information

Cardno South Trading as Ca ABN 95 001 14	ardno (NSW/ACT) Pty Ltd	Prepared for Project Name	Cardno NSW/ACT Pty Ltd Picton Town Centre – Various Locations – Existing Conditions
Ground Floor, PO Box 1285 Wollongong N	16 Burelli Street SW 2500	File Reference	RSA – Picton Town Centre – Various Locations – Existing Conditions
Telephone: 02	4228 4133	Job Reference	82018177-01
Facsimile: 02 4		Date	May 2018
southcoast@c www.cardno.c		Version Number	Ver 0
Author(s):	Antonio Villacorta Level 3 – Team Leader		
	Michael Renko Level 2 – Team Member	Effective Date	28/05/2018
Approved By:	Michael Renko Level 2 – Team Member	Date Approved:	28/05/2018

Document History

Version	Date	Description of Revision	Prepared by:	Reviewed by:
0	28/05/2018	DRAFT for internal Comment	Michael Renko	Antonio Villacorta
1	28/05/2018	Issue for Client Comment	Michael Renko	Tim Sullivan
2		Issue FINAL	TBC	TBC

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Project Summary

Project Number:	82018177-01
Final Report Date:	1/06/2018
Draft Report Date:	28/05/2018
Title of Audit:	Picton Town Centre – Various Locations – Existing Conditions
Location of Audit:	Picton Town Centre – Various Locations
Project Description:	The aim of this project is to assess several sites throughout the Picton Town Centre for potential road improvements.
Purpose of Audit:	The aim of this Road Safety Audit (RSA) is to assess the existing conditions.
State:	NSW
Stage of Audit:	Existing Conditions
Client Company:	Cardno NSW/ACT Pty Ltd
Client Contact:	Tim Sullivan
Client Phone:	9024 7055
Client Email:	Tim.Sullivan@Cardno.com.au
Audit Date:	21/05/2018
Audit Team:	Antonio Villacorta (Team Leader)
	Michael Renko (Team Member)



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Figures

Figure 3-1 Locality Plan

1



1 Project Description

The aim of this project is to assess the existing conditions of several sites throughout the Picton Town Centre for potential road improvements.

These sites include Colden Street/Margaret Street (Site 1), Argyle Street/Menangle Street (Site 2), Argyle Street/Barkers Lodge Road (Site 3) and Prince Street from Argyle Street to Menangle Street (Site 4).

2 Audit Stage

An Existing Conditions Stage Audit was carried out during a site visit of the various sites highlighted above during day and night time conditions on the 21st May 2018. At the time of the site visit weather was clear and traffic was moderate.

The audit was generally undertaken in accordance with 'TNSW Guidelines for Road Safety Audit Practices (2011)' and 'Austroads: Guide to Road Safety Part 6: Road Safety Audit (2009)'.

Findings of the audit are listed in Section 9, Tables 9.1 to Table 9.4.

3 Study Area

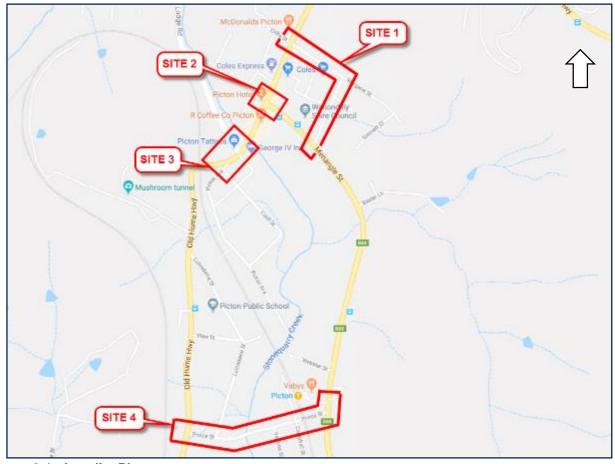


Figure 3-1 Locality Plan

Image sourced from Google Maps



4 Audit Team

The audit team and client details are shown in Table 4-1.

Table 4-1 Audit Team & Client Details

Role		
Client (Sponsor)	Cardno (NSW/ACT) Pty Ltd	
Client Contact	Tim Sullivan	Manager – Traffic and Transport
Client Email	Tim.Sullivan@Cardno.com.au	
Lead Auditor	Antonio Villacorta	Level 3 Auditor (RSA-02-0805)
Lead Auditor Email	Antonio.Arrollave@Kier.co.uk	
Team member	Michael Renko	Level 2 Auditor (RSA-02-1134)

5 Audit Program

The audit program details are shown in Table 5-1.

Table 5-1 Audit Program

Activity	Date	Attendees
Opening Meeting	14/05/2018	Michael Renko, Tim Sullivan
Site Inspection	21/05/2018	Michael Renko, Antonio Villacorta
Draft Report	29/05/2018	RSA Report (DRAFT for comment)
Completion Meeting	31/05/2018	Michael Renko, Tim Sullivan
Final Report	1/06/2018	RSA Report (Final for issue)

6 Background Information

As the time of the audit the sites highlighted in Figure 3-1 were observed as per existing conditions, no further background information was supplied to the audit team.

7 Exclusions

The existing bridge along Prince Street was excluded from the road safety audit, as such any safety hazard findings in this area have not been included.

The existing pedestrian crossing and school zone on Colden Street were excluded from the road safety audit, as such any safety hazard findings in this area have not been included.



8 Audit Risk Assessment Technique

For each of the safety issues identified, the level of risk with each has been determined. **Tables 8-1, 8-2 & 8-3** are extracted from Austroads: Guide to Road Safety Part 6: Road Safety Audit (2009) and have been used in the assessment of risk for this audit.

Table 8-1	Incident	Frequency
-----------	----------	-----------

Frequency	Description
Frequent	Once or more per week
Probable	Once or more per month
Occasional	Once or more per year
Improbable	Less often than once every year

Table 8-2 Incident Severity

Severity	Description	Examples
Catastrophic	Likely multiple deaths	 > High-speed, multi-vehicle crash on freeway. > Car runs into crowded bus stop. > Bus and petrol tanker collide. > Collapse of bridge or tunnel.
Serious	Likely death or serious injury	 > High or medium-speed vehicle/vehicle collision. > High or medium-speed collision with a fixed roadside object. > Pedestrian or cyclist struck by a car
Minor	Likely minor injury	 > Some low-speed vehicle collisions. > Cyclist falls from bicycle at low speed. > Left-turn rear-end crash in a slip lane.
Limited	Likely trivial injury or property damage only	 > Some low-speed vehicle collisions. > Pedestrian walks into object (no head injury). > Car reverses into post.

Table 8-3 Resulting Level of Risk Matrix

	Frequent	Probable	Occasional	Improbable
Catastrophic	Intolerable	Intolerable	Intolerable	High
Serious	Intolerable	Intolerable	High	Medium
Minor	Intolerable	High	Medium	Low
Limited	High	Medium	Low	Low



9 Audit Findings

Table 9-1 Colden Street / Margaret Street – Site 1

CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
1.1 Crossing Point – Limited Sight Margaret St/Argyle St Intersection	<text><text><text></text></text></text>	Occasional	Serious	High	Council to check crash history in this location and determine appropriate mitigation, if required.



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
1.2 Pedestrian Connectivity Menangle St/Colden St Intersection	<text></text>	Occasional	Serious	High	Council to consider opportunities to improve pedestrian crossings in this location as part of capital works program.



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
1.3 Existing Traffic Lanterns Intersection of Cliffe St/Argyle St/Margaret St	<text><text></text></text>	Improbable	Serious	Medium	Council / RMS to check crash history in this location and determine appropriate mitigation, if required.



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
1.4 Merge Lane Argyle St Southbound	There is a short merge lane southbound of the Argyle St/Cliffe St/Margaret St intersection. There is a risk motorists may fail to merge with adjacent traffic, resulting in a side-swipe or rear end collision. The frequency of the hazard is also increased as the existing sign 'Form One Lane' is obstructed by the large directory sign on approach of the merging lane.	Improbable	Serious	Medium	This is a fairly recent intersection upgrade. Presumably any design non- conformances relating to the departure merge lane were approved by RMS as part of the TCS sign-off process. Council / RMS to review TCS layout and placement of signs in this location and determine appropriate mitigation, if required.



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
1.5 Crossing Point – Desire Line Margaret St/Argyle St Intersection	<text><text></text></text>	Improbable	Serious	Medium	Council / RMS to review TCS layout and road geometry in this location and determine appropriate mitigation, if required.



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
1.6 Pedestrian Crossing Margaret St/Colden St Intersection	<text></text>	Improbable	Serious	Medium	Council to review landscaping, street furniture in this location and determine appropriate mitigation, if required.



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
1.7 Intersection Angle	There is an undesired intersection angle at the intersection of Menangle St and Colden St.	Occasional	Minor	Low	Council / RMS to review intersection layout and road
Intersection Angle Menangle St/Colden St Intersection	There is a risk that motorists turning from Colden St to Menangle St may endeavour to make the turn in an untimely manner or pass the holding line before making the turn due to limited sight of traffic on Menangle St resulting in a collision with through traffic. On site it was observed that vehicles frequently crossed the holding line to gain a better line of sight of through vehicles.				geometry in this location and determine appropriate mitigation, if required.
	Image sourced from Google Maps				



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
1.8 Turning Movements Cliffe St/Argyle St/Margaret St Intersection	It was observed on site that large vehicles turning onto Cliffe St from Argyle St frequently require a larger turning radius passing over the opposite traffic lane. There is a risk that large vehicles at this intersection may not have sufficient turning clearance resulting in a side-swipe or head-on collision with holding opposite traffic at the signalised intersection.	Improbable	Minor	Low	This is a fairly recent intersection upgrade. Presumably any design non- conformances relating to large vehicle swept paths were approved by RMS as part of the TCS sign-off process. Council / RMS to review TCS layout in this location and determine appropriate mitigation, if required.
1.9 Existing Signage Margaret St/Colden St Intersection	There is an existing Give way sign that is obstructed by vegetation. There is a risk a motorist may fail to appreciate the Give Way arrangement and enter the intersection in an untimely manner resulting in a collision with through traffic.	Improbable	Minor	Low	Council to review landscaping, street tree maintenance schedule in this location and determine appropriate mitigation, if required.



CAR S Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
Power Poles C Site Extents	<text></text>	Improbable	Minor	Low	Council / Utility Authority to review placement of power poles in this location and determine appropriate mitigation, if required.



Table 9-2 Argyle Street/Menangle – Site 2

CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
2.1 Wide Crossing Argyle St/Menangle St Intersection	There is a wide crossing on Argyle Street fitting up to 4 lanes of traffic simultaneously. There is a risk that pedestrians crossing the road will be exposed to high volumes of traffic with minimal visibility to the formalised pedestrian crossing resulting in a pedestrian-vehicle collision. On site it was observed that motorists on Argyle Street looking to turn onto Menangle Street often queued over the crossing, obstructing other motorists of being able to see pedestrians crossing the road with right of way behind the queued vehicles. The frequency of the hazards above are also increased by the observed high traffic volumes and different turning manoeuvres occurring simultaneously at this intersection.	Probable	Serious	Intolerable	Council / RMS to check crash history in this location and determine appropriate mitigation, if required.



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
2.2 Kerb Ramp Alignment Site Extents	<text></text>	Occasional	Serious	High	Council to consider opportunities to improve pedestrian crossings / accessibility improvements in this location as part of capital works program.



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
2.3 Concrete median Menangle Street	<text><text><text></text></text></text>	Improbable	Serious	Medium	Council to consider opportunities to improve pedestrian crossings / accessibility improvements in this location as part of capital works program.



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
2.4 Crossing Point – Desire Line Argyle St/Menangle St Intersection	<text><text></text></text>	Improbable	Serious	Medium	Council to consider opportunities to improve pedestrian crossings / accessibility improvements in this location as part of capital works program.



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
2.5 Turning Configurations Argyle St/Menangle St Intersection	There is insufficient delineation on Menangle Street heading into Argyle Street to identify right/left turning movements. There is a risk that a vehicle intending to turn left and another turning right may simultaneously queue in the lane with insufficient guidance/delineation and obstruct sight of traffic on Argyle Street resulting in a side-swipe incident, or collision with through traffic. The frequency of the hazards above are also increased by the high traffic volumes observed at this intersection.	Improbable	Minor	Low	Council to check crash history in this location and determine appropriate mitigation, if required.



Table 9-3 Argyle Street/Barkers Lodge Road.- Site 3

CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
3.1 Limited Sight Distance Argyle St/Barkers Lodge Road	<text><text><text></text></text></text>	Occasional	Serious	High	Council to check crash history in this location and determine appropriate mitigation, if required.



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
3.2 Commercial Property Access in Intersection Argyle St/Picton Ave	<text></text>	Occasional	Minor	Medium	Council to check crash history in this location and determine appropriate mitigation, if required.



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
3.3 Insufficient Intersection Treatments	It is noted that Argyle Street / Picton Avenue / Barkers Lodge Road / Crakanthorp Lane intersections present a numerous amount of turning manoeuvres within a short distance and limited sight distance of the through traffic on Argyle Street.	Occasional	Minor	Medium	Council to check crash history in this location and determine appropriate mitigation, if required.
Argyle Street, Picton Avenue, Barkers Lodge	There is insufficient delineation, linemarking and inadequate intersection traffic control off Picton Avenue to hold traffic before entering Argyle Street				
Road	There is a risk of long waiting queues, traffic conflicts and collisions for traffic at the unsignalised intersection of Picton Avenue and Argyle Street				
	enternette				



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
3.4 Lack of Delineation	There is insufficient delineation along Picton Avenue and its adjoining intersections Argyle Street and Coull Street. Picton Avenue also has deteriorated pavement conditions.	Improbable	Minor	Low	Council to check crash history in this location and determine appropriate mitigation, if
Picton Ave				required. Council to review site in respect of road maintenance priorities.	
					priorities.



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
3.5 Damaged Shoulder In Vicinity of Coull Street/Picton Ave Intersection	Coull Street shoulder northbound is damaged and in poor condition. There is a risk that during rainy days storm water may flush and wash debris into the Argyle Street intersection increasing the risk of slippery conditions. This could potentially increase the likelihood of vehicles losing control and collisions with opposite traffic.	Improbable	Minor	Low	Council to review site in respect of road maintenance priorities.



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
Reference/Location 3.6 Poor Delineation Argyle Street	Linemarking and delineation on Argyle Street north of Prince Street is not clear. Linemarking is faded. There is a risk that vehicles may confuse the lane configuration increasing the risk of head on collision with opposite traffic.	Improbable	Minor	Low	Council to review site in respect of road maintenance priorities.



Table 9-4 Prince Street from Argyle Street to Menangle Street.- Site 4

CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
4.1 Old Hume Highway / Prince Street Right Turn Old Hume Highway / Prince Street	<text><text></text></text>	Probable	Minor	High	Council to check crash history in this location and determine appropriate mitigation, if required.



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
4.2 Property Access	There are a number of property access points located close to or within the Argyle Street/Prince Street intersection.	Improbable	Serious	Medium	Council to check crash history in this location and determine
Locations Argyle St/Prince St Intersection	There is a risk that motorists entering these property access points will suddenly brake and reveal their intentions to turn without sufficient notice, resulting in a rear-end collision.				appropriate mitigation, if required.
	The frequency of the hazard is increased as driveway warning signage is currently located after the first driveway and not on approach of it.				



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
4.3 Short Stagger Distance Between Intersections Prince St/Menangle St/Station St	<text></text>	Occasional	Minor	Medium	Council to check crash history in this location and determine appropriate mitigation, if required.



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
4.4 Menangle St Shoulder	There is a shoulder on the northbound side of Menangle Street between Prince Street and Station Street without signage or on-street parking restrictions.	Improbable	Minor	Medium	Council to check crash history in this location and determine appropriate mitigation, if required.
Prince St/Menangle St/Station St	<text></text>				
4.5 Property Access Locations	There are a number of property access points located close to or within the Prince Street/Station St/Menangle Street intersections. There is a risk that motorists entering these property access points will suddenly brake and reveal their intentions to turn without sufficient notice,	Occasional	Minor	Medium	Council to check crash history in this location and determine appropriate mitigation, if required.
Prince St/Menangle St/Station St	resulting in a rear-end collision.				



CAR Reference/Location	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Managers Response
4.6 Widened Area Within Intersection Argyle St/Prince St Intersection	There is an existing widened area at the intersection of Argyle Street and Prince Street with inadequate delineation, its purpose is unclear. There is a risk that motorists may use it as a slip lane to turn into Prince Street with insufficient width resulting in side-swipe with adjacent traffic on Argyle Street.	Improbable	Minor	Low	Council to check crash history in this location and determine appropriate mitigation, if required.

10 Responding to the Audit Report

A project manager is under no obligation to accept the findings outlined in this audit report. This report simply provides the opportunity for the project manager to review potential problems highlighted by the auditors.

A formal road safety audit report should be responded to in writing.

It should be noted that this audit will be recorded on the NSW Register of Road Safety Auditors and the project manager should expect email notification from the register to confirm the audit has been carried out.

11 Formal Statement

We, the undersigned, declare that we have reviewed the material and data listed in this report and identified the safety and operational deficiencies above.

It should be noted that while every effort has been made to identify potential safety hazards, no guarantee could be made that every deficiency has been identified.

We recommend that points of concern be investigated and necessary corrective actions are undertaken.

Antonio Villacorta Level 3 Road Safety Auditor Auditor ID (RSA-02-0805) Team Leader Michael Renko Level 2 Road Safety Auditor Auditor ID (RSA-02-1134) Team Member

APPENDIX



RSA – STRATEGIC DESIGN



Road Safety Audit Report

Picton Town Centre – Pre-Construction Strategic Design Stage Road Safety Audit

8201817701

Prepared for Cardno NSW / ACT Pty Ltd

22 June 2018







Contact Information

Document Information

Cardno (NSW/ACT) Pty Ltd	Prepared for	Cardno NSW / ACT Pty Ltd
ABN 95 001 145 035	Project Name	Picton Town Centre – Pre-
Level 9 - The Forum		Construction Strategic
203 Pacific Highway		Design Stage Road Safety
St Leonards NSW 2065		Audit
Australia	File Reference	82018177_Picton RSA
www.cardno.com		Strategic v02.docx
Phone +61 2 9496 7700	Job Reference	8201817701
Fax +61 2 9496 7748	Date	22 June 2018
	Version Number	2

Document History

Version	Effective Date	Description of Revision	Prepared by	Reviewed by
0	22/06/2018	Draft (internal review)	Hayden Calvey	Antonio Villacorta
1	22/06/2018	Draft (client issue)	Hayden Calvey	Antonio Villacorta
2	22/06/2018	Final (client issue)	Hayden Calvey	Antonio Villacorta

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1 **Project Summary**

Project Number:	8201817701
Final Report Date:	22/06/2018
Draft Report Date:	22/062018
Title of Audit:	Picton Town Centre – Pre-Construction Strategic Design Stage Road Safety Audit
Location of Audit:	Argyle Street, Menangle Street, Prince Street – Picton NSW
Project Description:	The aim of this project is to assess a number of locations and proposed intersection treatments within the Picton town centre.
Purpose of Audit:	The aim of this Road Safety Audit (RSA) is to assess the proposed strategic design drawings against vehicle and pedestrian safety
State:	NSW
Stage of Audit:	Pre-Construction Strategic Design Stage Road Safety Audit
Client Company:	Cardno NSW / ACT Pty Ltd
Client Contact:	Tim Sullivan
Client Phone:	9024 7055
Client Email:	Tim.sullivan@cardno.com.au
Audit Date:	20/06/2018
Audit Team:	Antonio Villacorta (Auditor Level 3)
	Hayden Calvey (Auditor Level 2)



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Figure 4-1 Areas of Audit

2



2 **Project Description**

The aim of this project is to assess a number of locations and proposed intersection treatments within the Picton town centre.

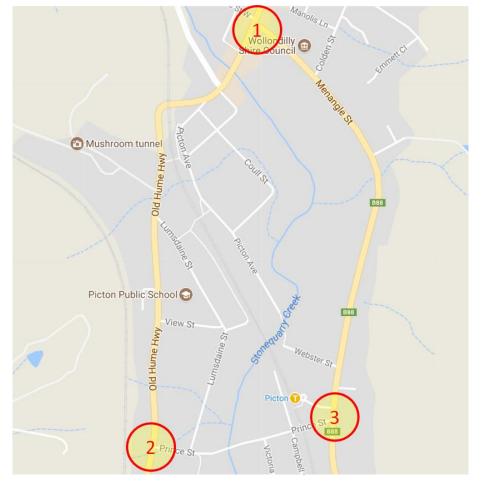
These sites include Argyle Street/Menangle Street (Site 1), Argyle Street/Prince Street (Site 2) and Menangle Street / Prince Street (Site 3).

3 Audit Stage

A pre-construction strategic design stage audit was carried out through desktop review of plans (as detailed in **Section 7**) on 20 June 2018. An existing stage audit was previously carried out on 21 May 2018.

The audit was generally undertaken in accordance with 'TNSW Guidelines for Road Safety Audit Practices (2011)' and 'Austroads: Guide to Road Safety Part 6: Road Safety Audit (2009)'.

4 Study Area



The focus of the study is limited to the three study sites as shown in Figure 4-1.

Figure 4-1 Areas of Audit



5 Audit Team

The audit team and client details are shown in Table 5-1.

Table 5-1 Audit Team & Client Details				
Role				
Client	Cardno NSW / ACT Pty Ltd			
Client Email Tim.sullivan@cardno.com.au				
Lead Auditor	Antonio Villacorta (RSA-02-0805)			
Team MembersHayden Calvey (RSA-02-0754)				
Contact for enquiries	Hayden.calvey@cardno.com.au			

6 Audit Program

The audit program details are shown in Table 6-1.

Table 6-1 Audit Program		
Activity	Date	
Desktop Audit	20/06/2018	Antonio Villacorta, Hayden Calvey
Draft Report issued	22/06/2018	Draft Report
Review/Closing Meeting	22/06/2018	Project Manager Comments
Final Report Issued	22/06/2018	Final Report

7 Exclusions

The following list identifies items excluded from the audit process:

- > Street lighting was not part of the audit
- > Pavement and drainage design were not part of this audit
- > Cross and longitudinal sections were not provided to the audit team
- > Swept paths for large vehicles over 8.8m length were not provided
- > Underground and overhead utilities were not provided



8 **Reference Documents**

The following reference documentation are the focus of the audit.

Table 8-1 List of Plans

Name	Reference	Date
Road Safety Audit Existing Conditions, Picton Town Centre – Various Locations – Existing Conditions	8201817701 Cardno Report	June 2018
Preliminary Argyle and Menangle Street and Menangle and Colden Street Intersection Layout Plan	82018177-01-SK001	08/06/2018
Preliminary Argyle and Menangle Street Intersection Vehicle Paths	82018177-01-SK002	08/06/2018
Preliminary Menangle and Prince Street Intersection Plan	82018177-01-SK003	08/06/2018
Preliminary Menangle and Prince Street Intersection Vehicle Paths Sheet 1 of 2	82018177-01-SK004	08/06/2018
Preliminary Menangle and Prince Street Intersection Vehicle Paths Sheet 2 of 2	82018177-01-SK005	08/06/2018
Preliminary Argyle and Prince Street Intersection Layout Plan	82018177-01-SK006	08/06/2018
Preliminary Argyle and Prince Street Intersection Vehicle Paths	82018177-01-SK007	08/06/2018

9 Audit Risk Assessment Technique

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For each of the safety issues identified, the level of risk with each has been determined. **Table 9-1**, **Table 9-2** and **Table 9-3** are extracted from Austroads: Guide to Road Safety Part 6: Road Safety Audit (2009) and have been used in the assessment of risk for this audit.

Table 9-1	Incident Frequency	
	Frequency	Description
	Frequent	Once or more per week
	Probable	Once or more per month
Occasional Once or more per year		Once or more per year
	Improbable	Less often than once every year

Table 9-2 Incident Severity

Severity	Description	Examples				
Catastrophic	Likely multiple deaths	 > High-speed, multi-vehicle crash on freeway. > Car runs into crowded bus stop. > Bus and petrol tanker collide. > Collapse of bridge or tunnel. 				
Serious	Likely death or serious injury	 > High or medium-speed vehicle/vehicle collision. > High or medium-speed collision with a fixed roadside object. > Pedestrian or cyclist struck by a car 				
Minor	Likely minor injury	 > Some low-speed vehicle collisions. > Cyclist falls from bicycle at low speed. > Left-turn rear-end crash in a slip lane. 				
Limited	Likely trivial injury or property damage only	 > Some low-speed vehicle collisions. > Pedestrian walks into object (no head injury). > Car reverses into post. 				

Table 9-3 Resulting Level of Risk Matrix

	Frequent	Probable	Occasional	Improbable
Catastrophic	Intolerable	Intolerable	Intolerable	High
Serious	Intolerable	Intolerable	High	Medium
Minor	Intolerable	High	Medium	Low
Limited	High	Medium	Low	Low



10 Audit Findings

CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
1.01	There is a wide crossing on Argyle Street fitting up to 4 lanes of traffic simultaneously.	Occasional	Serious	High	Outside of project scope. Zebra crossing relocation to be addressed by Council as a separate project.
	There is a risk that pedestrians crossing the road will be exposed to high volumes of traffic with minimal visibility to the formalised pedestrian crossing resulting in a pedestrian- vehicle collision.				
1.02	The proposed "No Right Turn" regulatory sign appears to be positioned on the concrete median in front of the existing "Stop" sign. There is a risk of signage blockage.	Improbable	Serious	Medium	Comment to be passed to design team. No Right Turn signage placement to be finalised at concept design stage.
	Location of sign far from the intersection may not be suitable to inform drivers of the movement restrictions. Vehicles could potentially continue turning right into Argyle Street increasing the risk of traffic conflicts and the already identify issues with pedestrians at the pedestrian crossing.				



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	NRT sign is positioned in front of Stop sign DP566894 PROPOSED R2-8(R) PROPOSED R2-8(R) EXISTING R1-1 TO REMAIN				
1.03	There is only one "No Right Turn" sign provided, approximately 14m (2 car lengths) back from the existing stop line.	Improbable	Serious	Medium	Comment to be passed to design team. No Right Turn signage to be finalised at concept design stage. It
	There is a risk that the limited number of signs and inappropriate location may result in approach vehicles not adequately observe the movement restriction and proceed to undertake a prohibit manoeuvre, resulting in a potential crash at the intersection.				should be noted that the proposal includes additional signage at Colden St to give right turners early warning.



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	Single NRT sign is positioned well behind the stop line. DP563577 2 DP56 PREFLACED WITH DRECTION TO THMOORBARGO ONLY 1 198867 1 198867 1 198067 1 19906 1990 1990 1990 1990 1990 1990 1				
1.04	The proposed "No Right Turn" is enforced by signage only, with no physical treatment. There is a risk that approach vehicles may undertake this prohibited manoeuvre resulting in a potential crash at the intersection.	Improbable	Serious	Medium	Physical medians to prohibit the right turn are not feasible due to the need to facilitate straight ahead movements and the offset geometry of the intersection. It is expected that the proportion of right turners ignoring the right turn ban will diminish over time as drivers get used to the changed traffic conditions.



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	PPEGAGET PPEGAG				
1.05	Due to the existing alignment of the intersection, it is unclear if access from Menangle Street (eastern approach) to Menangle Street West is intended to be prohibited as a result of the proposed "No Right Turn" The 'through' movement may be confused for drivers at the intersections as may appears to conflict with the proposed banned right turn movement.	Note Only			Comment to be passed to design team. Additional signage to be considered at concept design stage.



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	2 DP745335 EVENTSOLUTION SIGN TO BE TREPLICED WITH DIRECTION TO THE HONORRANGO ONLY 1 19867 1 19867 1 1 1000 1 1 1000 1 1 1000 100000 10000 1000 1000 1000 10000 1000 1000 1000000				
1.06	The swept path assessment adopts an 8.8m Medium Rigid Vehicle (MRV) however it is likely the intersection caters for general access vehicles (vehicles up to and including 19m semi-trailers) for turning movements.	Note Only			Comment to be passed to design team. Swept paths to be updated using 12.5m service vehicle as design vehicle and 19.5m semi- trailer as check vehicle. It should be noted that no physical changes are proposed to the road geometry in this location.
1.07	The displaced right turn traffic volume may result in capacity issues at other intersection locations (e.g. Argyle Street / Cliff Street), warrant the need to review existing pedestrian facilities (e.g. Colden Street) or review access to commercial properties on the north-western side of Argyle Street	Note Only			An Aimsun model has been used to confirm acceptable impacts at adjacent intersections up to 2026. Council to consider opportunities to improve pedestrian crossings as part of capital works program. Deliveries to commercial premises on the west side of Argyle St could potentially be done via Walton Street, with precise arrangements to be determined as part of stakeholder consultation prior



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
					to implementation of the changed traffic conditions.

Table 10-2 Audit Findings – Argyle Street / Prince Street Site 2

CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
2.01	The advance direction sign indicates two approach lanes on the southbound traffic lanes starting from the sign itself however does not display the requirement to merge before the intersection and then performing the fork diverge	Probable		The merge from two lanes to one lane SB is to facilitate an acceleration lane further south for left turners exiting Prince St. The	
	There is a risk that approach vehicles may disregard the merge requirement resulting in vehicle to vehicle conflict and potential side-swipe crash types.				microsimulation modelling showed that this free flow left turn from Prince St was required to maintain acceptable network performance in
	In addition, it is not clear to the audit team the purpose of the linemarking treatments.				the future.
	Internation of the source of t				The intent is for left turners into Prince St to stay in the kerbside lane The only vehicles required to merge are those wishing to continue straight (towards Tahmoor).
					Chevrons in the kerbside lane have been removed and merge arrows replaced with left turn arrows for final issue.
	7 DEVENDO FORMANDO E				Exact linemarking and signage to be resolved at concept design.
2.02	The requirement for the merge area on the northern approach is unclear to the audit team.	Probable	Minor	High	See response to 2.01 above.



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	There is a risk that approach vehicles may disregard the merge requirement resulting in vehicle to vehicle conflict and potential side-swipe crash types.				
	23 DP1107464 Unclear why vehicles are directed to merge				
2.03	The priority and purpose of the proposed intersection is unclear to the audit team. The right turn movement into Prince Street is proposed as a "Stop" control according to the proposed linemarking however the left turn into Prince Street presents as a continuous movement although is somewhat separated from the intersection. There is a risk that the confusing priorities at the intersection may result in potential near-side crashes.	Occasional	Serious	High	Give way and stop line marking corrected for final issue of drawings.



Road Safety Audit Report Picton Town Centre – Pre-Construction Strategic Design Stage Road Safety Audit

CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	27 DP1107464 PCPOGED BE LINE MARNING TO TE INTO EXERTING TO TE INTO EXERCISE TO TE INTO EXERCI				
2.04	The right turn to Argyle Street from Prince Street is proposed to be under "Stop" control based on the proposed linemarking however no "Stop" sign is proposed. I There is a risk that due to the inadequate signage an approaching vehicle may disregard the intersection control resulting in near and / or far-side crashes.	Improbable	Serious	Medium	The drawings only show new signage. The existing stop sign will be retained.



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	27 DP1197484 PROPOSED BILLINE MARKING MILLING MILLING SCIENCES 2122 2122 2122 2122 2122 2122 2122 21				
2.05	The intersection control applicable to the southern approach on Argyle Street is unclear to the audit team. The southern approach has a "Stop" control proposed for the right turn movement only, with the through (northbound) movement appearing to be a continuous movement. There is inadequate signage / warning of this intersection control and may result in confusion as to who is to stop and giveway. There is a risk that this atypical intersection control may result in abrupt braking resulting in rear-end crashes.	Occasional	Serious	High	Give way and stop line marking corrected for final issue of drawings.



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	27 DP1107A64 PROPOSED ELINE MARKING TO TE INTO EXISTING ELINE MARKING TO T				
2.06	It is unclear how pedestrians are catered to cross Prince Street. Design drawings do not provide details of any pedestrian treatment or consideration in the design for the existing kerb ramps on Prince Street	Improbable	Serious	Medium	Comment referred to design team. Location of dropped kerbs to be addressed at concept design.
	There is a risk that pedestrians may store within the chevron pavement marking area to cross Prince Street. This may result in vehicle to pedestrian conflict and result in a pedestrian related crash type.				



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	27 DP1107464 PROPOSED B8 LKE MARKING TO TE INTO EXISTING B1 LKE MARKING TO B1 D1 D1 D1 D1 D1 D1 D1 D1 D1 D1 D1 D1 D1				
2.07	It appears that the proposed widening of the intersection would require property boundary adjustments, residential driveway demolitions and potentially construction of retaining walls.	Improbable	Minor	Low	Comment referred to design team. Private driveway adjustments to be considered at concept design.
	There is risk that adjusted driveways will be steep to cater for the existing road level resulting in emerging from driveway crash type.				
	In addition it is not clear how the access to the two new property driveways would be retained				
2.08	There is on-street parking permitted along Argyle Street. There are no proposed parking restrictions within the proposed northbound through lane or the southbound lanes	Improbable	Minor	Low	Comment referred to design team. Parking restrictions to be considered at concept design. It should be noted
	There is a risk that parking may hinder / obstruct the intersection capacity and traffic flow which can lead to congestion, resulting in rear-end crash types.				that the proposed scheme does not exacerbate the likelihood or severity of the identified risk.
2.09	The continuous left turn slip lane from Prince Street into Argyle Street south proposes a C1 linemarking treatment. It is not clear to the audit team the purposes of the crossing	Improbable	Minor	Low	C1 line marking corrected for final issue of drawings.



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	merging linemarking as the two southbound lanes are not merging at any location nearby				
	There is a risk that southbound vehicles along Argyle Street may merge into the kerbside lane which is interpreted as the continuous lane for left turn traffic. This may result in side swipe crash types.				
	27 DP1107464 DP1107464 DP1107464 DP1107464 DP1107464 DP1107464 DP1107464 DP1107464 DP1107464 DP1107464 DP1107464 DP201413 DP2014 DP201413 DP201413 DP201413 DP201413 DP201413 DP201413				
2.10	The taper to divert vehicles to the left turn lane after the merge to the right painted chevron appears too short to allow vehicles for a smooth transition to the left turn lane.	Improbable	Serious	Medium	See response to 2.01 above.
	Vehicles may not have adequate time and space to merge back to the left lane increasing the risk of side and read end collisions.				



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	PROPOSED BB I INTO EXISTING I MIN. 40m				
2.11	There is an existing power pole at the southeast corner of Prince Street and Argyle Street. It appears that the proposed left turn lane would clash with the existing utility pole. There is a risk that clearance to the power pole may not be achieved increasing the risk of vehicles hitting the pole	Improbable	Serious	Medium	Power pole relocation to be addressed at concept design.
2.12	It is not clear to the audit team the purpose of the proposed bi-directional painted chevron for traffic travelling in the same direction. It appears an incorrect chevron pavement marking has been adopted to split the left and through lane movements on the northern approach. There is a risk that drivers may be confused with the painted marking increasing the risk of side collisions when attempting to navigate back to the through traffic lane	Occasional	Minor	Medium	Chevron line marking corrected for final issue of drawings.



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	MIN. 40m E5 E5 GRGYLE STREET CCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCC				
2.13	The existing intersection arrangement is noted to encourage vehicles on Prince Street to reduce speed and give way to the through traffic on Argyle Street. The proposed arrangement may encourage vehicles to maintain the travel speed entering the intersections The left turn lane from Prince Street into Argyle Street south appears to have inadequate geometry for the travel speed that may be performed when entering Argyle Street. The may increase the risk of vehicles losing control and crossing over the through traffic lane. This issue is reinforce with the proposed crossing linemarking C1 line off the left turn	Occasional	Minor	Medium	Precise kerb geometry to be addressed at concept design.
2.14	Design drawings do not provide details of any pavement treatment on Argyle Street for the proposed left turn lane into Prince Street. Typical or detailed cross sections were not provided to the audit team. The pavement conditions of the proposed slip left turn on Argyle Street present severe damages and differences in levels to the existing through lanes.	Occasional	Minor	Medium	Pavement and cross section details not required at strategic design stage. To be addressed at concept design.



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	There is a risk that usage of this lane may continue deteriorating the already damaged conditions of the lane.				
	In additional the difference of road levels may increasing the risk of losing control and mounting the kerb				
2.15	Design drawings provide details of swept path for vehicles up to 8.8m length.	Improbable	Minor	Low	This is an existing issue and it not within the scope of work for this project.
	It is not clear to the audit team is larger vehicles are allowed to enter Prince Street.				
	There are no regulatory signs to inform drivers of the low bridge clearance and maximum loading (5t) on Argyle Street. There is a risk that large vehicle may attempt to use the Prince Street used the proposed dedicated turning lanes.				
2.16	It is not clear to the audit team the purpose of the through traffic painted arrows. It is not a common practice in New South Wales nor adopted by Roads and Maritime	Note Only			This linemarking was adopted to help delineate the traffic movements on the northern approach. Linemarking to be finalised at concept design.
2.17	No swept paths have been provided for the right turn bay on the southern approach	Note Only			This right turn movement is unaffected by the proposed design, therefore the swept path is not included.
2.18	Cross sections were not provided to the audit team.	Note Only			Cross section details not required at strategic design stage. To be addressed at concept design.
	It is unclear to the audit team propose road levels, location of the crown and storm water management at the intersection				
2.19	The complexity of the proposed intersection may require suitable and sufficient directional and regulatory signage to advice drivers of the traffic conditions and facilitate navigation through the intersection.	Note Only			Given the non-standard nature of the intersection, it is recommended that a further road safety audit is undertaken at concept design stage.
	The information provided to the audit team was limited. A complete assessment of the intersection cannot be undertaken at this stage.				



Table 10-3 Audit Findings – Menangle Street / Prince Street Site 3

CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
3.01	It is unclear to the audit team if the existing footpath will be reinstated.	Occasional	Serious	High	Design amended for final issue, allowing footpath to be retained.
	There is a risk that pedestrians are forced to use the road as access resulting in vehicle to pedestrian conflict and pedestrian crash types				
	BB (15m) PCW PCW PCW PCW PCW PCW PCW PCW				
3.02	Menangle Street, northbound and southbound, currently have priority as the major road. The introduction of traffic signals will require these movements to stop and giveway to others resulting in changed traffic conditions.	Improbable	obable Serious Medium	Medium	This is no different to any other new set of traffic signals in NSW. Appropriate advance warning signage, lighting etc to be provided prior to the traffic signals being commissioned.
	There is a risk that motorists will be unfamiliar with the new intersection control type resulting in crashes at the intersection				



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response	
3.03	The active traffic lane is now positioned closer to the kerbside and existing utility poles. These utility poles are within the clearzone for the posted speed limit of 60km/h.	Improbable	Serious	Medium	Power pole locations to be reviewed during concept design.	
	There is a risk that vehicles may collide with the fixed roadside hazard resulting in injury.					
	Approximate location of existing utility poles					
3.04	Alignment / width of the proposed kerb ramps appear inadequate for the pedestrian crossing. There is a risk that pedestrians, including mobility impaired, are misdirected at the crossing point resulting in slip / trip falls.	Improbable	Limited	Low	Dropped kerbs are 2.0m wide, as per RMS Traffic Signal Design Guide Appendix D Section 1.5.	



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
3.05	<text><text></text></text>	Improbable	Serious	Medium	There are no footpath facilities on the east side of Menangle St. Recommend discussing with RMS the removal of this pedestrian crossing at concept design.
3.06	The existing driveway access for property 212 Menangle Street is within the uncontrolled area of the intersection. There is risk that entering / exiting vehicles may undertake manoeuvring (forward or reverse) that conflicts with the signal phasing and other vehicles travelling through the intersection resulting in crashes at the intersection	Occasional	Minor	Medium	Comment referred to design team. Private driveway adjustments to be considered at concept design.



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	Proposed intersection stop lines				
3.07	The turn arrow pavement marking spacing appears to be atypical and indicates that two right turn bays are functioning.	Occasional	Minor	Medium	Comment referred to design team. To be resolved at concept design.
	Vehicles awaiting to turn into Station Street may be overtaken by vehicle seeking to turn into Prince Street resulting in potential side swipe and rear end crash types.				



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	Overtaking manoeuvre				
3.08	There are no parking regulations around the intersection. There is a risk that parking may hinder / obstruct the intersection capacity and traffic flow that can lead to congestion, resulting in rear-end crash types.	Improbable	Minor	Low	Parking restrictions to be added at concept design.
3.09	There is likely to parked vehicles on the inside corner (left hand bend) where vehicles are required to shift laterally to the right. There is a risk that the conflicting road geometry and vehicle movement in conjunction with kerbside parking may result in crashes with parked vehicles.	Improbable	Minor	Low	Referred to design team. Linemarking, design extents and parking restrictions to be resolved at concept design.



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	Visual alignment of road				
3.10	There is inadequate width between the proposed central median and the kerb in the southbound direction to accommodate broken down vehicles based on the proposed length of the concrete median.	Improbable	Minor	Low	Referred to design team.
	There is a risk that a broken down vehicle may obstruct traffic flow resulting in congestion and potential rear end crash types.				



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	Broken down vehicle obstructing traffic flow				
3.11	It is unclear top the audit team if appropriate sight lines to the signal lanterns has been achieved. There is a risk that vehicles with the unfamiliar new intersection treatment may not have clear visibility of the traffic signals increasing the risk to enter the intersection and collide with opposite traffic.	Improbable	Serious	Medium	Lantern sight distance assessment to be done at concept design stage.
3.12	There is an existing street pole at the southwest corner of Prince Street and Menangle Street. It appears that the proposed left turn lane widening into Prince Street would clash with the existing utility pole. There is a risk that clearance to the power pole may not be achieved increasing the risk of vehicles hitting the pole	Improbable	Serious	Medium	Power pole relocation to be resolved at concept design.
3.13	Design drawings provide details of swept path for vehicles up to 8.8m length. It is not clear to the audit team is larger vehicles are allowed to enter Prince Street.	Improbable	Minor	Low	This is an existing issue and it not within the scope of work for this project.



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
	There are no regulatory signs to inform drivers of the low bridge clearance and maximum loading (5t) on Menangle Street. There is a risk that large vehicle may attempt to use the Prince Street used the proposed signalised intersection.				
3.14	It is unclear what the lantern type and arrangements are proposed.	Note Only			To be addressed at concept design.
3.15	The swept path assessment adopts an 8.8m Medium Rigid Vehicle (MRV) however it is likely the intersection caters for general access vehicles (vehicles up to and including 19m semi-trailers) for turn movements	Note Only			The weight restriction on the bridge prevents large vehicles turning at this intersection.
3.16	It is unclear the purpose of a a "kink" in the proposed kerb alignment on the eastern side of Menangle Street.	Note Only			This kerb adjustment is to maintain a 5.0m carriageway width on the departure lane.
3.17	Design drawings do not provide details for the propose usage of the traffic lanes. It appears a significant number of on-street parking spaces would be removed.	Note Only			Parking restrictions to be resolved at concept design.
	There are no details of proposed parking restriction or parking operations along Menangle Street				



CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
3.18	It is unclear to the audit team any treatment to the storm water management at the intersection. There is an existing drainage pit at the south west corner of Prince Street and Menangle Street that appears to clash with the proposed left turn lane widenings	Note Only			Stormwater adjustments to be resolved at concept design.

Table 10-4 General Notes

CAR Reference	Safety Hazard Findings	Frequency	Severity	Level Of Risk	Project Manager Response
4.01	It is not clear to the audit team proposed or retained road levels. Cross sectional detail has not been provided	Note Only			To be resolved at concept design.
4.02	Pavement type details has not been provided.	Note Only			To be resolved at concept design.
4.03	Vertical and horizontal long sections have not been provided	Note Only			To be resolved at concept design.
4.04	Geometry details (e.g. median types, kerb types) have not been provided	Note Only			To be resolved at concept design.
4.05	Underground and overhead utility planes were not provided. There is a risk of utility clashes at the intersection with the introduction of the Roads and Maritime underground cabling and pavement widening.	Note Only			To be resolved at concept design.
4.06	Drainage plans have not been provided	Note Only			To be resolved at concept design.
4.07	Traffic Control Signal (TCS) design plans have not been provided	Note Only			To be resolved at concept design.

11 Responding to the Audit Report

A project manager is under no obligation to accept the findings outlined in this audit report. This report simply provides the opportunity for the project manager to review potential problems highlighted by the auditors.

A formal road safety audit report should be responded to in writing.

It should be noted that this audit will be recorded on the NSW Register of Road Safety Auditors and the project manager should expect email notification from the register to confirm the audit has been carried out.

12 Formal Statement

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We, the undersigned, declare that we have reviewed the design drawings listed in Section 8 Reference Documents in this report and identified the safety and operational deficiencies above.

It should be noted that while every effort has been made to identify potential safety hazards, no guarantee could be made that every deficiency has been identified.

We recommend that points of concern be investigated and necessary corrective actions are undertaken.

Antonio Villacorta Level 3 Road Safety Auditor Team Leader

Hayden Calvey Level 2 Road Safety Auditor Team Member

APPENDIX



SIDRA MOVEMENT SUMMARIES



Site: [Menangle St / Prince St AM 2 lane W]

Option 1

Signals - Fixed Time Isolated Cycle Time = 80 seconds (Optimum Cycle Time - Minimum Delay)

Move	ment Per	rformance -	Vehicle	es							
Mov	OD	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Menanag	le St									
1	L2	340	0.3	0.237	8.2	LOS A	3.6	25.3	0.29	0.66	51.6
2	T1	405	7.3	0.810	27.7	LOS B	14.7	109.3	0.85	0.85	41.2
Appro	ach	745	4.1	0.810	18.8	LOS B	14.7	109.3	0.60	0.77	45.4
North:	Menanagl	le St									
8	T1	717	8.2	0.776	17.5	LOS B	22.8	170.8	0.84	0.78	46.6
9	R2	60	1.8	0.436	46.7	LOS D	2.4	17.2	0.99	0.75	33.3
Appro	ach	777	7.7	0.776	19.8	LOS B	22.8	170.8	0.85	0.78	45.2
West:	Prince St										
10	L2	45	2.3	0.052	17.6	LOS B	0.9	6.7	0.56	0.68	45.5
12	R2	474	0.2	0.829	38.1	LOS C	19.2	134.8	0.97	0.93	36.1
Appro	ach	519	0.4	0.829	36.3	LOS C	19.2	134.8	0.93	0.91	36.8
All Vel	nicles	2041	4.5	0.829	23.6	LOS B	22.8	170.8	0.78	0.81	42.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians								
Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective	
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate	
		ped/h	sec		ped	m		per ped	
P3	North Full Crossing	11	23.3	LOS C	0.0	0.0	0.76	0.76	
P4	West Full Crossing	11	20.3	LOS C	0.0	0.0	0.71	0.71	
All Pe	destrians	21	21.8	LOS C			0.74	0.74	

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: [Menangle St / Prince St AM 2 lane W]

Option 1

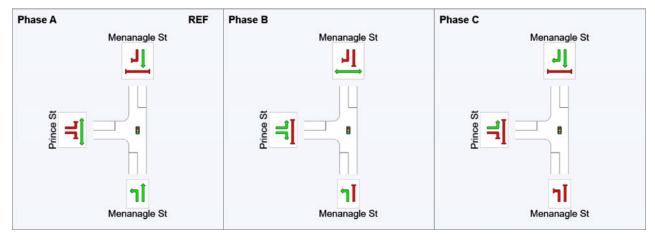
Signals - Fixed Time Isolated Cycle Time = 80 seconds (Optimum Cycle Time - Minimum Delay)

Phase Times determined by the program Phase Sequence: Two-Phase Reference Phase: Phase A Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

Phase Timing Results

Phase	Α	В	С
Phase Change Time (sec)	0	36	68
Green Time (sec)	30	26	6
Phase Time (sec)	36	32	12
Phase Split	45%	40%	15%

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase



Site: [Menangle St / Prince St PM 2 lane W]

Option 1

Signals - Fixed Time Isolated Cycle Time = 84 seconds (User-Given Phase Times)

Move	ment Per	formance -	Vehicle	s							
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/ł
South	Menanag	le St									
1	L2	561	0.2	0.353	6.8	LOS A	4.6	32.5	0.22	0.65	52.6
2	T1	622	8.6	0.971	58.9	LOS E	34.0	255.8	0.73	1.11	30.5
Appro	ach	1183	4.6	0.971	34.2	LOS C	34.0	255.8	0.49	0.89	38.1
North:	Menanagl	e St									
8	T1	399	10.3	0.333	6.8	LOS A	7.1	54.4	0.47	0.42	53.9
9	R2	65	1.6	0.995	80.6	LOS F	3.8	27.2	1.00	1.08	25.4
Appro	ach	464	9.1	0.995	17.2	LOS B	7.1	54.4	0.55	0.51	46.6
West:	Prince St										
10	L2	21	5.0	0.034	25.0	LOS B	0.6	4.1	0.69	0.68	41.6
12	R2	405	0.3	0.945	62.7	LOS E	22.4	156.8	1.00	1.08	29.1
Appro	ach	426	0.5	0.945	60.8	LOS E	22.4	156.8	0.98	1.06	29.5
All Vel	nicles	2074	4.8	0.995	35.9	LOS C	34.0	255.8	0.61	0.84	37.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians									
Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective		
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate		
		ped/h	sec		ped	m		per ped		
P3	North Full Crossing	11	30.0	LOS D	0.0	0.0	0.85	0.85		
P4	West Full Crossing	11	12.1	LOS B	0.0	0.0	0.54	0.54		
All Pe	destrians	21	21.0	LOS C			0.69	0.69		

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: [Menangle St / Prince St PM 2 lane W]

Option 1

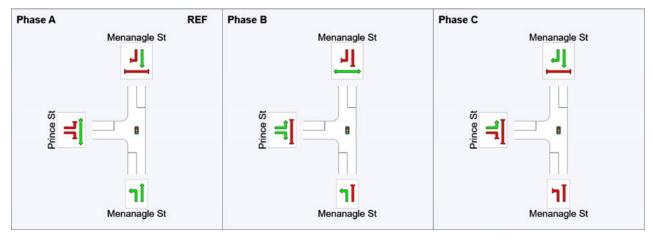
Signals - Fixed Time Isolated Cycle Time = 84 seconds (User-Given Phase Times)

Phase Times specified by the user Phase Sequence: Two-Phase Reference Phase: Phase A Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

Phase Timing Results

Phase	Α	В	С
Phase Change Time (sec)	0	49	75
Green Time (sec)	46	20	3
Phase Time (sec)	52	26	6
Phase Split	62%	31%	7%

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase



Site: [Menangle St / Prince St AM 1 lane W]

Option 2

Signals - Fixed Time Isolated Cycle Time = 85 seconds (Optimum Cycle Time - Minimum Delay)

Move	ment Per	formance -	Vehicle	s							
Mov	OD	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Menanag	le St									
1	L2	339	0.3	0.232	8.0	LOS A	3.6	25.1	0.28	0.66	51.7
2	T1	407	7.2	0.827	30.4	LOS C	16.0	118.7	0.85	0.87	39.9
Appro	ach	746	4.1	0.827	20.3	LOS B	16.0	118.7	0.59	0.77	44.6
North:	Menanagl	e St									
8	T1	687	8.6	0.757	17.6	LOS B	22.3	167.8	0.83	0.76	46.5
9	R2	58	1.8	0.447	49.5	LOS D	2.5	17.7	1.00	0.75	32.3
Appro	ach	745	8.1	0.757	20.1	LOS B	22.3	167.8	0.84	0.76	45.0
West:	Prince St										
10	L2	68	1.5	0.845	40.0	LOS C	23.8	167.1	0.99	0.95	35.6
12	R2	471	0.2	0.845	40.0	LOS C	23.8	167.1	0.99	0.95	35.5
Appro	ach	539	0.4	0.845	40.0	LOS C	23.8	167.1	0.99	0.95	35.5
All Vel	nicles	2031	4.6	0.845	25.4	LOS B	23.8	167.8	0.79	0.81	41.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians									
Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective		
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate		
		ped/h	sec		ped	m		per ped		
P3	North Full Crossing	11	23.4	LOS C	0.0	0.0	0.74	0.74		
P4	West Full Crossing	11	19.8	LOS B	0.0	0.0	0.68	0.68		
All Pe	destrians	21	21.6	LOS C			0.71	0.71		

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: [Menangle St / Prince St AM 1 lane W]

Option 2

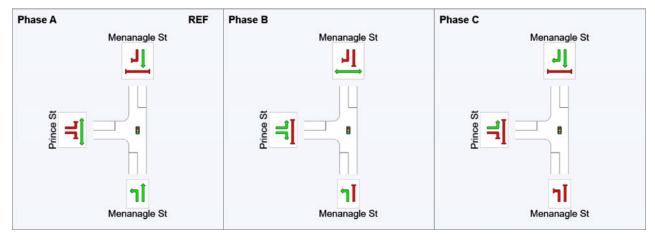
Signals - Fixed Time Isolated Cycle Time = 85 seconds (Optimum Cycle Time - Minimum Delay)

Phase Times determined by the program Phase Sequence: Two-Phase Reference Phase: Phase A Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

Phase Timing Results

Phase	Α	В	С
Phase Change Time (sec)	0	38	73
Green Time (sec)	32	29	6
Phase Time (sec)	38	35	12
Phase Split	45%	41%	14%

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase



Site: [Menangle St / Prince St PM 1 lane W]

Option 2

Signals - Fixed Time Isolated Cycle Time = 84 seconds (User-Given Phase Times)

Move	ment Per	formance -	Vehicle	s							
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South	: Menanag	le St									
1	L2	555	0.2	0.349	6.8	LOS A	4.6	31.9	0.22	0.65	52.6
2	T1	626	8.6	0.972	59.3	LOS E	34.4	258.6	0.74	1.12	30.3
Appro	ach	1181	4.6	0.972	34.6	LOS C	34.4	258.6	0.49	0.90	37.9
North:	Menanagl	e St									
8	T1	414	9.9	0.345	6.9	LOS A	7.5	56.8	0.48	0.42	53.9
9	R2	66	1.6	1.011	88.1	LOS F	4.1	29.3	1.00	1.11	24.0
Appro	ach	480	8.8	1.011	18.1	LOS B	7.5	56.8	0.55	0.52	46.0
West:	Prince St										
10	L2	24	4.3	0.907	53.8	LOS D	20.4	143.1	1.00	1.02	31.3
12	R2	378	0.3	0.907	53.8	LOS D	20.4	143.1	1.00	1.02	31.3
Appro	ach	402	0.5	0.907	53.8	LOS D	20.4	143.1	1.00	1.02	31.3
All Ve	nicles	2063	4.8	1.011	34.5	LOS C	34.4	258.6	0.61	0.83	37.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians									
Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective		
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate		
		ped/h	sec		ped	m		per ped		
P3	North Full Crossing	11	30.0	LOS D	0.0	0.0	0.85	0.85		
P4	West Full Crossing	11	11.0	LOS B	0.0	0.0	0.51	0.51		
All Pe	destrians	21	20.5	LOS C			0.68	0.68		

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: [Menangle St / Prince St PM 1 lane W]

Option 2

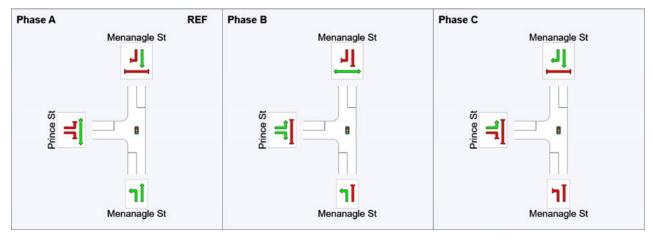
Signals - Fixed Time Isolated Cycle Time = 84 seconds (User-Given Phase Times)

Phase Times specified by the user Phase Sequence: Two-Phase Reference Phase: Phase A Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

Phase Timing Results

Phase	Α	В	С
Phase Change Time (sec)	0	49	75
Green Time (sec)	46	20	3
Phase Time (sec)	52	26	6
Phase Split	62%	31%	7%

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase



Site: Option [Argyle Street / Lumsdaine Street AM]

2026 AM

Signals - Fixed Time Isolated Cycle Time = 65 seconds (Optimum Cycle Time - Minimum Delay)

Move	ement <u>Pe</u>	rformance -	Vehicle	es							
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South	: Argyle St		70	V/C	300		Ven				KI1/11
1	L2	6	5.0	0.844	22.3	LOS B	29.7	216.7	0.87	0.89	45.9
2	T1	973	5.0	0.844	16.7	LOS B	29.7	216.7	0.87	0.89	47.1
3	R2	1	5.0	0.002	14.1	LOS A	0.0	0.1	0.50	0.60	47.5
Appro	bach	980	5.0	0.844	16.7	LOS B	29.7	216.7	0.86	0.89	47.1
East:	Lumsdaine	e Street									
4	L2	1	5.0	0.007	29.1	LOS C	0.1	0.4	0.83	0.57	41.0
5	T1	1	5.0	0.007	23.5	LOS B	0.1	0.4	0.83	0.57	41.9
6	R2	14	5.0	0.049	30.7	LOS C	0.4	2.7	0.86	0.68	38.9
Appro	bach	16	5.0	0.049	30.1	LOS C	0.4	2.7	0.86	0.67	39.2
North	: Argyle St	reet									
7	L2	42	5.0	0.285	11.9	LOS A	4.8	35.0	0.50	0.47	52.3
8	T1	569	5.0	0.285	7.0	LOS A	4.8	35.0	0.52	0.47	53.5
9	R2	6	5.0	0.285	13.3	LOS A	4.8	34.9	0.55	0.47	51.4
Appro	bach	618	5.0	0.285	7.4	LOS A	4.8	35.0	0.52	0.47	53.4
West:	New Road	d									
10	L2	13	5.0	0.071	30.9	LOS C	0.6	4.0	0.87	0.69	39.1
11	T1	1	5.0	0.071	25.3	LOS B	0.6	4.0	0.87	0.69	40.0
12	R2	6	5.0	0.071	30.9	LOS C	0.6	4.0	0.87	0.69	39.0
Appro	bach	20	5.0	0.071	30.6	LOS C	0.6	4.0	0.87	0.69	39.1
All Ve	hicles	1634	5.0	0.844	13.5	LOS A	29.7	216.7	0.73	0.73	49.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedestria	ans						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P1	South Full Crossing	105	26.9	LOS C	0.2	0.2	0.91	0.91
P2	East Full Crossing	105	7.9	LOS A	0.1	0.1	0.49	0.49
P3	North Full Crossing	53	26.8	LOS C	0.1	0.1	0.91	0.91
All Pe	destrians	263	19.3	LOS B			0.74	0.74

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: Option [Argyle Street / Lumsdaine Street AM]

2026 AM

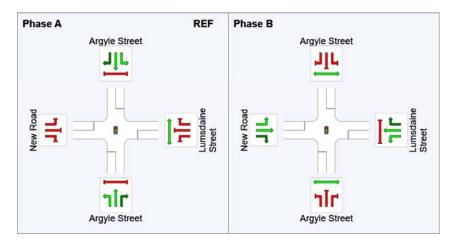
Signals - Fixed Time Isolated Cycle Time = 65 seconds (Optimum Cycle Time - Minimum Delay)

Phase Times determined by the program Phase Sequence: Two-Phase Reference Phase: Phase A Input Phase Sequence: A, B Output Phase Sequence: A, B

Phase Timing Results

Phase	Α	В
Phase Change Time (sec)	0	46
Green Time (sec)	40	13
Phase Time (sec)	46	19
Phase Split	71%	29%

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase



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Site: Option [Argyle Street / Lumsdaine Street PM]

2026 AM

Signals - Fixed Time Isolated Cycle Time = 65 seconds (User-Given Cycle Time)

Movement Performance - Vehicles											
Mov	OD	Demand	Flows	Deg.	Average	Level of	95% Back of Queue		Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Ocuth	. Annula Ol	veh/h	%	v/c	sec		veh	m		per veh	km/h
South: Argyle Stre											
1	L2	11	5.0	0.580	13.7	LOS A	12.7	93.1	0.64	0.58	51.4
2	T1	661	5.0	0.580	8.1	LOS A	12.7	93.1	0.64	0.58	52.9
3	R2	1	5.0	0.003	14.1	LOS A	0.0	0.1	0.50	0.60	47.5
Appro	bach	673	5.0	0.580	8.1	LOS A	12.7	93.1	0.64	0.58	52.9
East: Lumsdaine Street											
4	L2	1	5.0	0.004	30.9	LOS C	0.0	0.2	0.86	0.59	39.0
5	T1	1	5.0	0.018	23.8	LOS B	0.1	1.0	0.84	0.63	41.0
6	R2	4	5.0	0.018	29.4	LOS C	0.1	1.0	0.84	0.63	40.0
Appro	bach	6	5.0	0.018	28.7	LOS C	0.1	1.0	0.84	0.62	40.0
North	: Argyle St	reet									
7	L2	29	5.0	0.371	12.3	LOS A	6.7	49.2	0.53	0.49	52.2
8	T1	808	5.0	0.371	6.7	LOS A	6.7	49.2	0.53	0.48	53.9
9	R2	5	5.0	0.371	12.3	LOS A	6.6	48.1	0.53	0.47	52.2
Appro	bach	843	5.0	0.371	6.9	LOS A	6.7	49.2	0.53	0.48	53.8
West:	New Roa	d									
10	L2	2	5.0	0.031	30.5	LOS C	0.2	1.7	0.86	0.65	39.4
11	T1	1	5.0	0.031	24.9	LOS B	0.2	1.7	0.86	0.65	40.3
12	R2	5	5.0	0.031	30.5	LOS C	0.2	1.7	0.86	0.65	39.3
Approach		8	5.0	0.031	29.8	LOS C	0.2	1.7	0.86	0.65	39.5
All Ve	hicles	1531	5.0	0.580	7.7	LOS A	12.7	93.1	0.58	0.53	53.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

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Site: Option [Argyle Street / Lumsdaine Street PM]

2026 AM

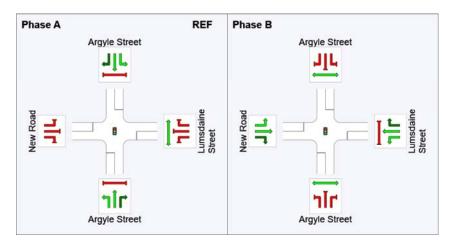
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