Reeves Creek Development

Traffic Impact Assessment

NA50613047

Prepared for Michael Brown Planning on behalf of Dartanyon Pty Ltd

July 2014





Contact Information

Cardno (NSW/ACT) Pty Ltd Trading as Cardno Ltd (ASX: CDD)

ABN 95 001 145 035

Level 9 – The Forum 203 Pacific Highway St Leonards NSW 2065

Telephone: (02) 9496 7700 Facsimile: (02) 9439 5170 International: +61 2 9496 7700

Sydney.traffic@cardno.com.au

www.cardno.com

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1 Introduction

1.1 Overview

Cardno has been commissioned by Michael Brown Planning on behalf of Dartanyon Pty Ltd to undertake a traffic and transport assessment to support a rezoning application for a residential lead land use development located in Picton, called Reeves Creek.

Once rezoned for residential land use it is anticipated that the proposed development will be undertaken in two separate stages. This assessment investigates the traffic and transport impacts pertaining to Stage 1 of the development which includes construction of 400 low to medium density residential dwellings.

The overall development Reeves Creek masterplan is presented in **Appendix A** of this report.

1.2 Report Content

This document considers the proposed Reeves Creek residential land use development and details the traffic and transport implications of the proposed residential lead rezoning application on the surrounding road network.

The remainder of the report is structured as follows:

- > **Section 2** provides details of the existing traffic and transport environment surrounding the proposed development site.
- > **Section 3** provides details of the proposed development.
- > Section 4 details the findings of the traffic assessment of the proposed development.
- > Section 5 addresses the issues raised by RMS (letter reference: STH13/00050) and Wollondilly Shire Council.
- > Section 6 provides a conclusion by summarising the findings and recommendations of the assessment.

1.3 Reference Documents / Data

The following documents have been referenced while undertaking the transport assessment discussed in this report:

- > Stone guarry Commercial Development Traffic Impact Assessment (Cardno, 2014);
- > Service Planning Guidelines (Ministry of Transport, June 2006);
- > Wollondilly Shire Council Development Control Plan (2011); and
- > Wollondilly Shire Council Tracks Model.

1.4 Correspondence with Wollondilly Shire Council

The proposed rezoning application and proposed scope of works for the traffic and transport assessment was discussed with Wollondilly Shire Council (contact: Dick Webb – Manager Infrastructure Planning)

The following scope of works was discussed and verbally agreed with the Council (please refer to **Appendix B** for email correspondence):

Utilise the Wollondilly Shire TRACKS model to undertake the traffic assessment for the Reeves Creek
rezoning application located in Picton. It is understand that the Council is looking to update the
TRACKS model. However the timeframe for this update has not been confirmed and would fall outside
the delivery timeframe of our project. Therefore, the use of the current model would be appropriate for
the purposes of this assessment.

- The methodology of our traffic assessment shall be consistent with that was used for the Stone Quarry rezoning application and therefore the forecast flow, anticipated development trip generation and assignment shall be obtained by utilising the TRACKS model.
- The intersections to be assessed are as follows (for the weekday AM and PM peak hour periods):
 - Site access / Menangle Street
 - o Argyle Street / Menangle Street
 - o Argyle Street / Margaret Street / Cliffe Street
- The design years that will be assessed are 2016 and 2026 for the 'Base' and 'Base plus Development' scenarios.
- The schemes / road upgrades that are committed and in the surrounding area are as follows:
 - Proposed signalisation of the Argyle Street / Margaret Street / Cliffe Steet intersection (open 2014).
 - Proposed roundabout at the Argyle Street / Regreme Road intersection (open 2016).
- The traffic assessment for the Reeves Creek development would not need to consider the traffic generated by the Stone Quarry rezoning application as this has not been approved and is not a committed development.

2 Existing Conditions

2.1 Surrounding Roads

The existing road network surrounding the development site consists of:

- > Menangle Street;
- > Margaret Street; and
- > Argyle Street (also known as Remembrance Drive).

The location of the land, which is the subject of the rezoning application, in the context of the surrounding road network is shown in **Figure 2-1** below.



Figure 2-1 Site Location

Background Source: <u>www.nearmap.com</u>

2.1.1 <u>Menangle Street</u>

Menangle Street is a State Road under the authority of the Roads and Maritime Services (RMS). The RTA (now RMS) NSW Road Management Arrangements (2008) defines State Roads as follows:

- 'State Roads are the major arterial links throughout NSW and within major urban areas. They are the principle traffic carrying and linking routes for the movement of people and goods within the Sydney, Newcastle, Wollongong and Central Coast urban areas and which connect between these urban centres, the major regions of the State and the major connections interstate.'
- 'The RTA takes responsibility for managing the primary traffic function of State Roads including funding
 and determining priorities, and regulates the activities of third parties on the road and access to
 adjoining land to promote road safety, traffic efficient and protect the road asset.'

Menangle Street intersects with Argyle Street to the north and becomes Picton Road at its intersection with Mineral Springs Road to the south. Menangle Street / Picton Road provide a link between Argyle Street and the Hume Motorway.

Menangle Street fronting the subject site comprises of a carriageway with one travel lane in each direction (separated by solid double barrier lines) and a posted speed limit of 60 km/hr.

2.1.2 Argyle Street

Argyle Street is classified as a Regional Road under the authority of the Council. The RTA (now RMS) NSW Road Management Arrangements (2008) defines Regional Roads as follows:

- 'Regional Road are routes of secondary importance between State Roads and Local Roads which together with the State Roads, provide the main connections to and between smaller towns and districts and perform a sub arterial functions in major urban areas.'
- 'Regional Roads are the responsibility of councils to fund, determine priorities and carry out works.'
- 'Regional Roads are eligible for annual assistance grants from the State Government in recognition of their relative importance.'
- 'Councils also apply other sources of funding to works on Regional Roads including local rates, developer contributions and funding from the Federal Government.'

Argyle Street provides access to local shops with kerbside parking provision on either side of the carriageway.

Argyle Street comprises of a carriageway with one travel lane in each direction and a posted speed limit of 50km/h.

2.1.3 Margaret Street

Margaret Street is classified as a Local Road under the authority of the Council. The RTA (now RMS) NSW Road Management Arrangements (2008) defines Local Roads as follows:

- 'Local Roads comprise the remaining council controlled roads which provide for local circulation and access.'
- 'Local Roads are the responsibility of Councils to fund, determine priorities and carry out works.'
- 'The State Government provides only limited assistance under special programs eg urban Bus Routes.'
- 'The Federal Government has a long standing role in providing road funds to councils. It provides annual financial assistance grants to councils that include a significant identified roads component.'

Margaret Street has a posted speed limit of 50km/h and one lane of travel in each direction with kerbside parking provision provided on either side of the carriageway.

2.2 Key Intersections

The following are considered to be key intersections as highlighted in **Figure 2-2**:

- 1. Argyle Street / Margaret Street / Cliffe Street.
- 2. Argyle Street / Menangle Street.
- 3. Site Access / Menangle Street.

Traffic volumes for the surrounding road network were obtained from the Wollondilly Shire Council TRACKS model and were used in order to determine the future year baseline flows.

The key intersections were modelled, using software package SIDRA 6.0, in order to establish the anticipated traffic impacts of the proposed development on operation performance.



Figure 2-2 Location of Key Intersections

Background Source: www.nearmap.com

The following sections provide details of the two Argyle Street intersections alongside their respective configurations that were modelled using SIDRA 6.0 software.

2.2.1 <u>Argyle Street / Margaret Street / Cliffe Street Intersection</u>

The intersection of Argyle Street / Margaret Street / Cliffe Street consists of four arm approaches which are currently controlled by Stop conditions on Margaret Street and Cliffe Street. Wollondilly Shire council has confirmed that this intersection will be upgraded to a signalised control by the end of 2014. As such, this intersection has been modelled as a signalised intersection in all of the future year scenarios analysed. It is noted that the Council confirmed that they would not be able to provide layout plans of the proposed signalised intersection due to protection restrictions. Through conversations with the Council assumptions have therefore been made in relation to the intersection layout. **Figure 2-3** below illustrates the assumed signalised configuration of this intersection used in the SIDRA analysis.

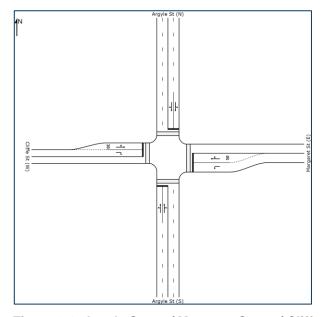


Figure 2-3 Argyle Street / Margaret Street / Cliffe Street Intersection Layout

2.2.2 <u>Argyle Street / Menangle Street Intersection</u>

This is a four-way intersection which is currently controlled by Stop conditions on the Menangle Street approaches. **Figure 2-4** illustrates the current configuration of this intersection used in the SIDRA analysis.

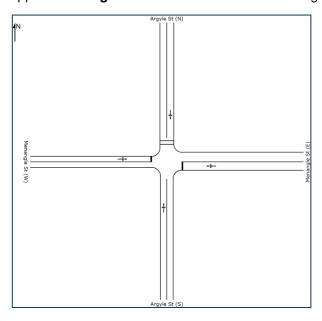


Figure 2-4 Argyle Street / Menangle Street Intersection

2.3 Public Transport

2.3.1 Bus Routes

There are five bus routes which operate in vicinity of the proposed development along Argyle Street. These bus routes and their service frequencies are summarised in **Table 2-1** below. All of the bus services listed in the table below are operated by Picton Busline.

Table 2-1 Bus Services

Route #		Weekday	Services	
	Route Description	AM Peak (7- 10am)	PM Peak (4- 7pm)	Saturday Services
900	Picton to Narellan & Campbelltown	1	0	2
911	Picton to Buxton & Bargo	2	0	0
912	Picton to Bargo & Yanderra	2	6	9
914	Picton to Buxton v Estonian Village	0	0	0

2.3.2 Bus Stops

There are two bus stops located within a 500 metre walking catchment of the proposed development. The locations of bus stops within the vicinity of the proposed development are presented in **Figure 2-5** below.



Figure 2-5 Bus Stop Locations

Background Source: www.nearmap.com

2.3.3 <u>Rail</u>

The proposed development site is approximately 1 km walking distance from Picton Railway Station. Picton Railway Station is located on the Southern Highlands Line of the Intercity TrainLink network, as indicated in **Figure 2-7** Train Network Map —Picton Station

Travel time from the station to the proposed development is an approximate 12 minute walk, 5 minute cycle or 2 minute drive. The travel path to the train station is presented in **Figure 2-8.**

7.



Figure 2-7 Train Network Map –Picton Station

Travel time from the station to the proposed development is an approximate 12 minute walk, 5 minute cycle or 2 minute drive. The travel path to the train station is presented in **Figure 2-8**.



Figure 2-8 Train Station Location

Background Source: www.nearmap.com

3 Proposed Development

3.1 Reeves Creek Development Masterplan

The proposed residential lead rezoning application is related to the existing vacant land bound by private residential properties, vacant land and Argyle Street to the north and east; Menangle Street and private residential properties to the west and private residential properties and land to the south.

Once the rezoning application has been approved, the overall residential lead development is anticipated to be undertaken in two separate stages and will be subject to individual Development Application (DA) submissions. This assessment investigates the traffic and transport impacts pertaining to Stage 1 of the development.

The boundary of Stage 1 development and the proposed internal development zones are illustrated in **Figure 3-1** below. The respective areas of each of these zones and their proposed development yields are summarised in **Table 3-1** below.

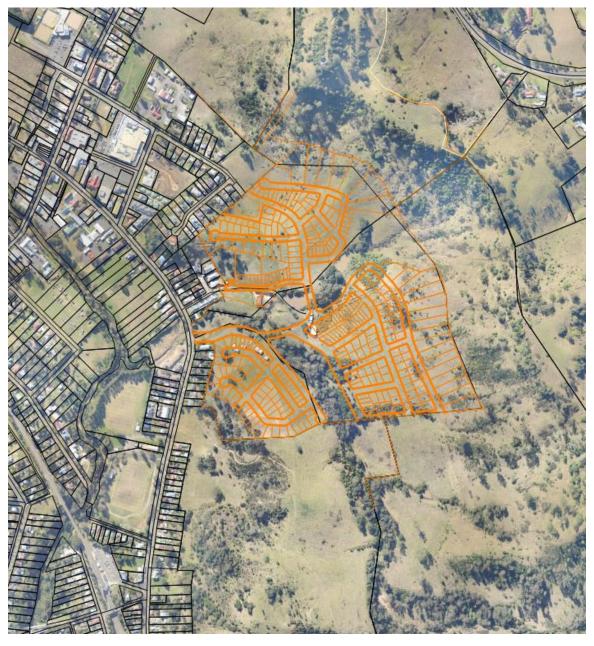


Figure 3-1 Reeves Creek Development – Stage 1

3.2 Development Yield

Stage 1 of the proposed development is anticipated to consist of low to medium density residential, environmental conservation and public recreation land use areas. **Table 3-1** below outlines the proposed uses and their respective areas and development yields within Stage 1 of the Reeves Creek development.

Table 3-1 Proposed Reeves Creek Stage 1 Internal Development Zones

Proposed Zones	Area (Gross Hectares)	Gross Density Range	Yield (Number of Dwellings)
Environmental Conservation- E2	4.3	-	0
Low Density Residential – R2	22	10-15 Dwellings/Hectare	220
Medium Density Residential – R3	9.9	18-22 Dwellings/Hectare	180
Public Recreation – RE1	2.6	-	0
Totals	38.8		400

3.3 Internal Road Hierarchy

The internal road hierarchy within the Reeves Creek masterplan area shall be based on the function and anticipated traffic volumes. The cross sections for the internal roads to the Reeves Creek development shall accord with Council requirements (reference shall be made to Wollondilly Shire Council Subdivision & Engineering Standards – Design Specifications). At this early stage, the anticipated internal road hierarchy is illustrated in **Figure 3-2** below.

Given that it is intended for the proposed collector roads to accommodate buses, these roads will be constructed to comprise a 13m minimum carriageway width with provision for bus stop spaces at the required intervals.

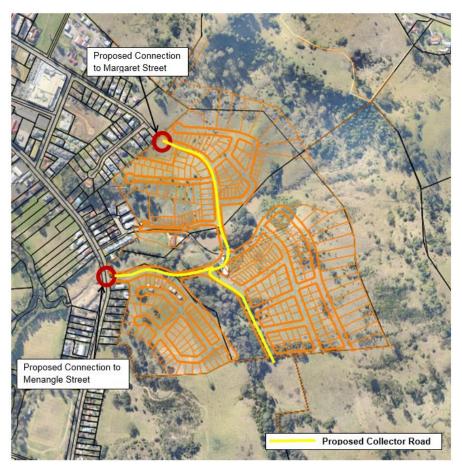


Figure 3-2 Internal Road Hierarchy

3.4 Site Access

Access to the site will be provided via both Margaret Street and Menangle Street as discussed below:

- Margaret Street Access Margaret Street extends south east from its intersection with Emmett Close.
 This extension of Margaret Street will be used as a connection point to the proposed internal collector
 road within the subject development. It is noted that the Argyle Street / Margaret Street / Cliffe Steet
 intersection is to be upgraded to a signals by late 2014.
- Menangle Street Access It is proposed to upgrade the existing access located to the east side of Menangle Street, in order to join the proposed internal collector road within the subject development.
 The proposed access configuration with Menangle Street is discussed in further detail below.

The locations of the above mentioned connections are shown in Figure 3-2.

3.4.4 Menangle Street Access

Given the current arrangement and location of the Menangle Street access point, it is acknowledged that the existing access arrangement shall need to be upgraded in order to safely and operationally accommodate the forecast traffic flows generated by the proposed residential lead development. The following sub-sections will outline the constraints of the existing situation and further investigate a suitable access upgrade.

3.4.4.1 Priority controlled access with auxiliary / deceleration left turn lane and channelised right turn on Menangle Street

The requirement for auxiliary lanes on Menangle Street, at the site access location, has been investigated as a part of this study. The figure below illustrates the forecast number of vehicle turning movements at the Menangle Street / Site Access intersection during the PM peak hour for 2026 baseline plus development scenario (worst case scenario).

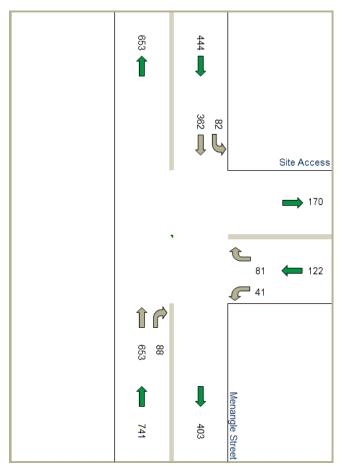


Figure 3-3 Forecast Traffic Volumes at Menangle Street/Site Access Intersection during the PM Peak for 2026 Base plus Development Scenario

Based on the turning traffic volumes illustrated in the figure above, the requirements for turn treatments on Menangle Street can be established using the chart presented in **Figure 3-3** below (excerpt from Austroads Guide to Road Design Part 4A).

For the vehicles turning left from Menangle Street onto the Site;

- These vehicles will cause delays to the through southbound movements along Menangle Street (i.e. 362 vehicles/hour).
- Total of 82 vehicles will turn left into the site from Menangle Street during the PM peak hour.
- These traffic volumes, when plotted on the chart below (warrants for turn treatments), indicate a
 requirement for a short auxiliary lane (AUL(S)) to accommodate the left turning vehicles.

For the vehicles turning right from Menangle Street onto the Site;

- These vehicles will cause delays to both northbound and southbound through vehicles along Menangle Street (i.e.1,015 vehicles/hour).
- Total of 88 vehicles will turn right into the site from Menangle Street during the PM peak hour.
- These traffic volumes, when plotted on the chart below (warrants for turn treatments) indicate a requirement for a channelised right turn (CHR) lane.

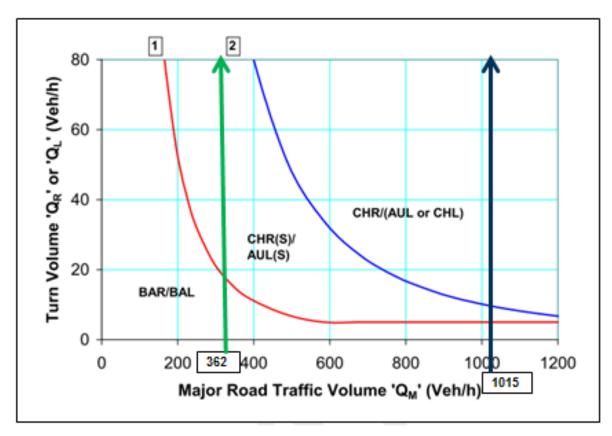


Figure 3-3 Warrants for Turn Treatments on the Major Road (Austroads Guide Part 4A)

Therefore, it is proposed to upgrade the primary site access to a priority controlled access with auxiliary / deceleration left turn lane and channelised right turn on Menangle Street.

From the turn treatments established above, the subject intersection has been assessed in SIDRA software to model the performance with the development traffic. The following figure illustrates the configuration used to model the performance of this intersection.

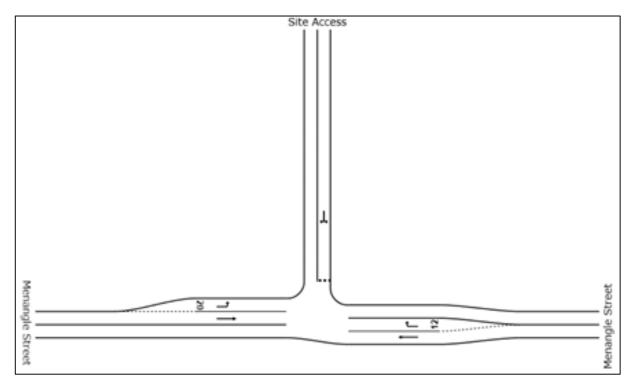


Figure 3-4 Menangle Street Site Access Configuration

3.4.4.2 Menangle Street/Site Access – Safe Intersection Sight Distance

It is acknowledged that the site access point intersects the outside of the horizontal curvature of Menangle Street. Therefore the Safe Intersection Sight Distance (SISD) has been assessed at this location to ensure that adequate visibility is provided between the motorists on conflicting movements.

SISD is the minimum distance which should be provided on the major road at any intersection and the SISD model presented in Austroads Guide Part 4A (Unsignalised and Signalised Intersections) is presented in **Figure 3-5** below.

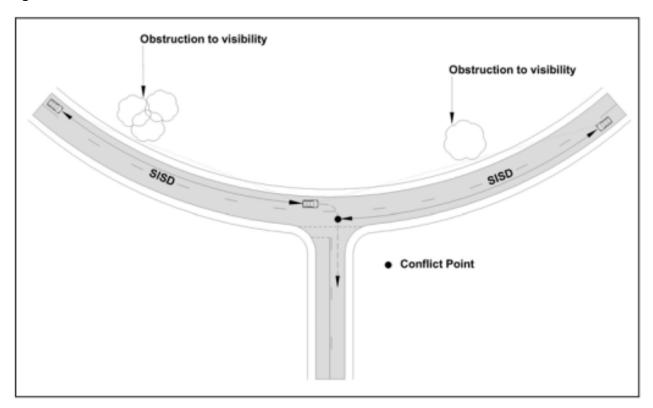


Figure 3-6 Figure 3-5 SISD model for minor roads intersection on the outside of the horizontal curves - Austroads Guide to Road Design Part 4A

Referring to Table 3.2 in Austroads Guide Part 4A, the SISD applicable for the site access on Menangle Street is 123m (based on 60 km/h design speed along Menangle Street and a reaction time of 2.0 seconds). The following figure outlines the SISD model applied to this scenario.



Figure 3-7 The SISD Envelops for Vehicles Approaching the Site Access Point

From the SISD envelop illustrated in the figure above, it is evident that the vehicles approaching the site access point can foresee any potential conflicts with a clear sight envelop of 123m (with no permanent obstructions present within the sight triangles). Therefore, this sight distance, along Menangle Street, is deemed adequate to cater for the access point to the proposed development.

3.4.4.3 Menangle Street/Site Access – Minimum Gap Sight Distance

The Minimum Gap Sight Distance (MGSD) is based on distances corresponding to the critical acceptance gap that drivers are prepared to accept when undertaking a crossing or turning manoeuvre at intersections. The figure below illustrates the sight distance to a through vehicle from a vehicle turning left. Based on the

Austroads Guide the minimum gap sight distance required is 83m. This requirement can be sufficiently met at the site access intersection with Menangle Street.

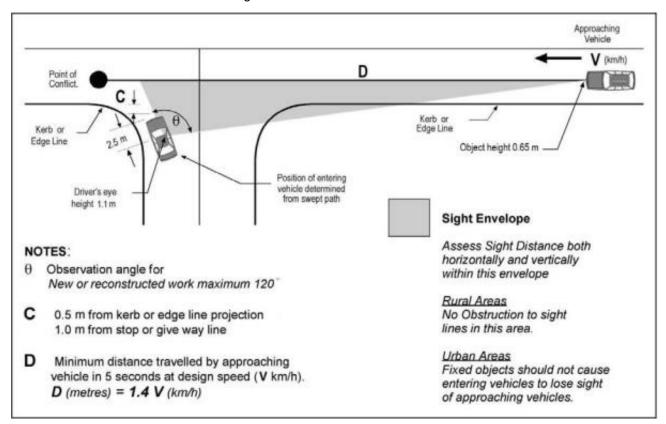


Figure 3-8 Sight distance to a through vehicle from a vehicle turning left (MGSD) – Austroads Guide to Road Design Part 4A

3.5 Off-Street Parking Requirements

Part 2.3.7 of the Wollondilly Shire Council Development Control Plan (2011) sets out a parking requirement for the provision of at least 2 vehicle spaces behind the front building line for dwelling houses. As such, for the proposed 400 dwellings as part of Stage 1 development, provision shall be made for a total of 800 off-street car parking spaces. Sustainable Travel

Accessible, frequent and direct public transport options that encourage future Reeves Creek residents to reduce their trips taken by private vehicle will reduce the traffic generation from the site and reduce the greenhouse gas emissions associated with car travel. A public transport network will also ensure that all residents have equitable opportunity to access employment and recreational opportunities. Public transport should be viewed as convenient and attractive to the estate's new residents.

As the Reeves Creek masterplan area is undeveloped there exists opportunities to improve the existing transport networks. Analysis and recommendations to address any identified deficiencies shall be undertaken as part of the DA submission process. Key considerations shall be:

- > Provision of a permeable network for pedestrians and cyclists (footpath, cycle lanes, shared pathways).
- > Prominent and high quality walking and cycling facilities to connect residents to external network.
- > In line with TfNSW guidance, 80%-90% of dwellings should be within 400-500m of a bus route (a 5 minute walk).
- > The extension of existing or new bus routes into the development to serve residents of Reeves Creek being aware that route deviations to minimise walking times or access to low patronage generators will impact on travel time for most of the passengers which will reduce the attractiveness of the service. Any future bus routes within the site should be in line with the Collector Road network.
- > To facilitate the use of bus services it is important to provide accessible bus stops to ensure services are available to all users and do not restrict the potential demand for patronage.

- > Recognising that some services may be less frequent, lower order bus stop and shelter facilities are required to be provided for rural bus routes. However as these services operate less frequently, shelter and seating should be provided where appropriate.
- > Ensuring that gradients of roads and associated infrastructure (such as pathways) are within standards and do not present barriers to future use by pedestrians, cyclists, bus services and passengers.
- > Provision of safe and efficient links to the external network and main trip attractors such as Picton Railway Station.

4 Traffic Impact Assessment

4.1 Baseline Intersection Performance

4.1.4 Overview of Assessment

The operating performance of the Argyle Street / Margaret Street / Cliffe Street, Argyle Street / Menangle Street intersections as well as the site access onto Menangle Road was assessed using SIDRA 6.0 intersection modelling software package to determine the Degree of Saturation (DoS), Average Vehicle Delay (AVD) in seconds, Level of Service (LoS) and 95th percentile queues. The following scenarios were considered in this assessment for a typical weekday AM and PM peak hour:

- > 2016 Baseline Scenario;
- > 2016 Baseline with Development Scenario;
- > 2026 Baseline Scenario; and
- > 2026 Baseline with Development Scenario.

The RMS Guide to Traffic Generating Developments (Version 2.2, 2002) provides a guide in assessing level of service for various intersections. An extract of the guide is shown below in **Table 4-1** and highlights the key indicators in evaluating intersection performance.

Table 4-1 Level of Service Summary

LOS	Traffic Signal / Roundabout	Give Way / Stop Sign / T-Junction Control
Α	Good operation	Good operation
В	Good operation, with acceptable delays and spare capacity	Acceptable delays and spare capacity
С	Satisfactory	Satisfactory, but accident study required
D	Operating near capacity	Near capacity and accident study required
E	At capacity; at signals, incidents will cause excessive delays	At capacity, requires other control mode
F	Unsatisfactory and requires additional capacity. Roundabouts require other control mode	Unsatisfactory and requires additional capacity. Roundabouts require other control mode

For signalised intersections, the overall level of service should be considered. For roundabouts and priority controlled intersections (sign control) individual lanes should be analysed.

The Average Vehicle Delay (AVD) provides a measure of performance, relating average delay to the level of service, and should be taken as a guide only. The average delay measures level of service based on delay per second per vehicle.

The RMS Guide to Traffic Generating Developments identifies the key criteria in assessing the level of service based on average delays and can be seen in **Table 4-2** below.

Table 4-2 Level of Service Average Vehicle Delay

LOS	Average Delay per Vehicle (sec/veh)
Α	< 14
В	15 to 28
С	29 to 42
D	43 to 56
E	57 to 70
F	> 70

Another form of operational measurement is to assess the Degree of Saturation (DoS) of individual intersections. An intersection at DoS of up to 0.8 is considered satisfactory. Intersections are reaching capacity as the DoS approaches 0.9, with queue lengths increasing and extended delays.

4.1.5 <u>Intersection Performance and Results</u>

Traffic analysis of the baseline scenario was undertaken using the SIDRA Intersection 6.0 software package, to assess the future base intersection operational performance prior to the construction of the proposed development.

The traffic volumes for this analysis are based upon the Wollondilly Shire Council TRACKS model obtained. These traffic volumes, obtained from TRACKS model outputs for years 2016 and 2026, are presented in **Appendix D**. The detailed SIDRA assessment outputs, for all scenarios assessed, are presented in **Appendix E**.

4.1.6 2016 Baseline Scenario

Based upon the intersection layouts indicated in **Section 2.2**, the results of the analysis for the year 2016 baseline scenario with *SIDRA Intersection 6.0* is presented in **Table 4-3** below.

Table 4-3 2016 Baseline Scenario SIDRA Summary

Intersection	Intersection Control	AM Peak				PM Peak		
		Degree of Saturation	Delay (s)	Level of Service	Degree of Saturation	Delay (s)	Level of Service	
Argyle Street / Cliffe Street / Margaret Street	Signalised	0.449	21.5	В	0.462	26.3	В	
Argyle Street / Menangle Street	Give Way	1.197	448	F	1.306	584.4	F	

The results outlined above reveal satisfactory performance of Argyle Street/Cliffe Street/Margaret Street intersection. However, the Argyle Street/Menangle Street intersection indicated unsatisfactory operations, with its capacity reached by 2016 (operating at a Level of Service F). As such, infrastructure upgrade is necessary by 2016 to the Argyle Street/Menangle Street intersection, in order for the intersection to perform at a satisfactory operational performance under the baseline scenario.

The recommended infrastructure upgrades to the Argyle Street/Menangle Street intersection and its operational performance is shown below in **Figure 4-Figure 4-11** and **Table 4-1** respectively. The modelling results of this intersection, with the proposed upgrades, indicate satisfactory intersection operational performance.

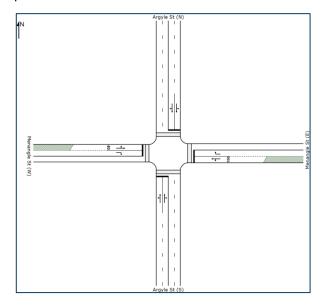


Figure 4-11 2016 Baseline Scenario SIDRA Layout (with Upgrades) for Argyle Street / Menangle Street Intersection

Table 4-4 2016 Baseline Scenario SIDRA Summary with Upgrades

				<i>,</i> - g-			
	Intersection		AM Peak			PM Peak	
Intersection	Control	Degree of Saturation	Delay (s)	Level of Service	Degree of Saturation	Delay (s)	Level of Service
Argyle Street / Menangle Street	Signalised	0.496	24	В	0.558	28.4	В

The results of the intersection analysis indicate that with the proposed upgrades the intersections will operate with small delays at a Level of Service B during both AM and PM peak periods.

4.1.7 <u>2026 Baseline Scenario</u>

The traffic volumes for 2026 baseline scenario were obtained from the Wollondilly Shire Council TRACKS model obtained. These traffic volumes, obtained from TRACKS model outputs, are presented in **Appendix D**.

The following table outlines the SIDRA modelling outputs for the key intersections for the 2026 baseline scenario.

Table 2-6 2026 Baseline Scenario SIDRA Summary

Intersection	Intersection	AM Peak			PM Peak		
	Control	Degree of Saturation	Delay (s)	Level of Service	Degree of Saturation	Delay (s)	Level of Service
Argyle Street / Cliffe Street / Margaret Street	Signalised	0.625	23.8	В	0.89	36	С
Argyle Street / Menangle Street	Signalised	0.771	28.5	С	0.669	27.2	В

It is to be noted that the Argyle Street / Menangle Street intersection has been assessed as a signalised intersection with the same layout adopted in the 2016 baseline scenario with the proposed upgrade (as this intersection fails at the existing layout in 2016 baseline scenario).

The baseline 2026 results indicate that the intersections generally perform at a satisfactory level, with the Argyle Street / Menangle Street intersection operating at Levels of Service C and B respectively for both AM and PM peak periods. The intersection has a degree of saturation below 0.9 and therefore the delays and queuing are generally satisfactory.

The Argyle Street / Cliffe Street / Margaret Street intersection indicates satisfactory operation during both AM and PM peak periods with a Level of Service A with acceptable delays and gueues.

4.2 Development Traffic Generation

In order to assess the impact of the proposed development on the external road network, the proposed dwelling numbers (400 dwellings) associated with the development were added to the representative zone in the TRACKS model which calculated the trip generation and distributed and assigned trips accordingly.

4.3 Intersection Operational Performance

The proposed numbers of residential dwellings were introduced into the base TRACKS model in order to assess the impact of the proposed development. The resulting intersection traffic volumes (provided in **Appendix D**) were assessed using the *SIDRA Intersection 6.0* modelling software. The detailed SIDRA modelling outputs, for each scenario assessed, are presented in **Appendix E**.

4.3.4 <u>2016 Baseline plus Development Scenario Intersection Results</u>

Table 4-1 below outlines the intersection operational performance for the 2016 base plus development scenario.

Table 4-1 2016 Baseline plus Development Scenario SIDRA Summary with Upgrades

	=	=			-	_		
	Intersection	AM Peak				PM Peak		
Intersection	Control	Degree of Saturation	Delay (s)	Level of Service	Degree of Saturation	Delay (s)	Level of Service	
Argyle Street / Cliffe Street / Margaret Street	Signalised	0.467	24	В	0.456	26.4	В	
Argyle Street / Menangle Street	Signalised	0.525	24.8	D	0.629	31.3	С	
Site Access / Menangle Street	Priority	0.034	11.8	В	0.140	11.2	В	

As indicated in the table above, the Level of Service of the Argyle Street/Cliffe Street/Margaret Street intersection has dropped from LoS A in the baseline scenario to LoS B in the baseline with development scenario, for both AM and PM peak periods. This is due to the marginal increase in the average delay associated with the proposed development traffic. However, the degree of saturation of this intersection, during both peak periods, remains below 0.8 and is therefore the operations are considered satisfactory.

The Argyle Street/Menangle Street intersection operates at LoS B during the AM peak period in both the baseline scenario and the baseline with development traffic scenario. However, the PM peak period operations indicate that the Level of Service has been reduced from LoS B in the baseline scenario to LoS C in the baseline with development traffic scenario. This reduction in the Level of Service is due to the marginal increase of the average delay associated with the development traffic. However, this intersection still operates with a degree of saturation well below 0.8, during both peak periods. Therefore, it is unlikely to cause excessive queues and delays.

The proposed upgraded Site Access onto Menangle Street is anticipated to operate well within capacity with a Los B for both AM and PM peak periods.

4.3.1 2026 Baseline plus Development Scenario Intersection Results

Table 4-2 below outlines the intersection operational performance for the 2026 base plus development scenario.

Table 4-2 2026 Base plus Development Model SIDRA Summary with Upgrades

	Intersection	AM Peak				PM Peak		
Intersection	Control	Degree of Saturation	Delay (s)	Level of Service	Degree of Saturation	Delay (s)	Level of Service	
Argyle Street / Cliffe Street / Margaret Street	Signalised	0.700	26.5	В	0.908	37.8	С	
Argyle Street / Menangle Street	Signalised	0.815	29.2	С	0.708	28.5	С	
Site Access / Menangle Street	Priority	0.052	14.7	В	0.182	13.3	В	

From the intersection operational performance results outlined in the table above, it is evident that the Level of Service of the Argyle Street/Cliffe Street/Margaret Street intersection has dropped from LoS B in the 2026 baseline scenario to LoS C in the baseline with development scenario, for the AM peak period.

The Argyle Street/Menangle Street intersection operates with a Level of Service C, which is retained from the baseline scenario, for both AM and PM peak periods. The degree of saturation for this intersection remains below 0.9 for both peak periods assessed. As such, this intersection operates satisfactorily with acceptable delays and queuing in 2026 with development traffic.

The proposed upgraded Site Access onto Menangle Street is anticipated to continue to operate well within capacity with a Los B for both AM and PM peak periods.

5 Response to Comments by Council and RMS

The following sections summarises the responses to specific concerns raised by the Council and RMS.

5.1 Responses to Comments by Wollondilly Shire Council

As a part of the Menangle Street Planning Proposal - Specialist Studies Requirements, Wollondilly Shire Council has raised Traffic and Transport related concerns. The responses to each of these concerns are summarised in **Table 5-1** below.

Table 5-1 Summary of Concerns Raised by Wollondilly Shire Council

Concern raised by Wollondilly Shire Council Response The traffic and transport assessment should give consideration to the impacts on the local road network of the proposed and potential development over the whole property, utilising; This traffic assessment has been carried out by Council's strategic network model (TRACKS) to adding the proposed development yields on to the determine traffic distributions to and from the site; and existing Wollondilly Shire TRACKS model and subsequently modelling each intersection in Intersection modelling using SIDRA to assess the impact SIDRA. of the proposed new road on the local road network; and Specific consideration being given to the treatment type for the proposed subdivision access road junction with the classified road network. Review and address the RMS requirements outlined in their Section 5.2 below. submission letter. Identify suitable infrastructure required to ameliorate traffic and safety impacts associated with the likely future development of the See Conclusions section of this report subject land. Identification of pedestrian, cyclist and public transport Further details, including proposed cross sections infrastructure needs required to service of the internal road network, will be provided during the proposed development. Identify the timing of the infrastructure and appropriate planning her details, including proposed cross sections of the mechanism to ensure the infrastructure is provided. internal road network, will be provided during DA stage.

5.2 Responses to Comments by RMS

It is acknowledged that RMS has raised traffic and parking concerns related to the subject development in their letter referenced: STH13/00050. The following table will outline and provide responses to each of these issues.

Table 5-2 Summary of Concerns Raised by RMS

Concern raised by RMS	Response
A Traffic Impact Study should be prepared in accordance with Table 2.1 of the RTA Guide to Traffic Generating Developments.	This traffic assessment has been undertaken to address specific issues pertaining to traffic and transport issues arising from the subject development.
Individual intersections should also be modelled in SIDRA where TRACKS has been used for the network model. This would be required for any proposed access road junctions with the classified roads concerned and potentially any other classified	Individual intersections have been modelled in SIDRA software and the results are presented in Section 4 of this report.

road junctions in the vicinity that are likely to be adversely impacted by the traffic generated by the subject development. The treatment type for the proposed subdivision access road junction with the classified road network would need to be determined based on the intersection modelling. The selection of treatment types should also consider other constraints on the Refer to Section 3.4.4 and Figure 3.4. classified road/s such as the speed environment and road safety, and the land available within the road reserve to create the junctions. Electronic copies of all modelling undertaken to support the planning proposal should be provided to Council and RMS for review. Noted. RMS' specific requirements for the treatment type of the proposed access road junction with Menangle Street (or Remembrance Driveway) will be provided following its assessment of the Traffic Impact Study and associated intersection modelling. The sight distance being significantly constrained due to the road No connection is proposed directly to curvature along the Remembrance Driveway at the boundary of Remembrance Driveway. the subject site. The existing road environment and nature of development on the northern side of Remembrance Driveway makes this stretch of No connection has been proposed on to Remembrance Drive conductive to a 100km/h environment and Remembrance Driveway; as such no change in therefore RMS is unlikely to consider any proposed reduction of speed limit is required. the sign posted speed limit as compliance and enforcement issues are likely to result. Consideration should be given to the size of the largest vehicle that will be required to access the site, including waste collection vehicles. Remembrance Driveway at this location is an approved No new intersections have been proposed along B-Double route, therefore, any proposed intersection with Remembrance Driveway. Remembrance Drive will need to cater for vehicles of this size to undertake the through movements. Consideration should be given to the impact of any loss of on-The minor loss of on-street public parking spaces street parking and impacts on adjoining private access points on along Menangle Street due to the proposed site Menangle Street as a result of road or transport infrastructure access point can be relocated elsewhere along improvements necessitated by future development of the site. the same road. It is also noted that given the residential frontage on Menangle Street, the onstreet parking demand is anticipated to be low. The RMS preferred strategy is to deny access to the classified road where alternative local road access is available. Where this No connection proposed is directly to is not feasible, RMS is likely to restrict access to left in/left out as Remembrance Driveway. traffic volumes increase and right turning movements begin to compromise road safety and traffic efficiency. The internal road network within the site would need to allow for future subdivided lots to gain access to the internal subdivision Noted. This is incorporated within the road roads to avoid any need for additional connections to Menangle design. Street or Remembrance Driveway. RMS notes that the proposed zoning of R2 and R3 allows for the development of Child Care Centre and Educational Establishments. For safety reasons Council should ensure that The planning proposal for this site indicates only any new Child Care Centres or Schools are prohibited where such residential developments. properties have a direct frontage to a classified road by including a clause in the LEP. RMS would not support a development application within the subject site for this type of land use if the

site had frontage/a boundary to Menangle Street or Remembrance Driveway.

RMS strongly supports development with the potential to reduce car dependency and encourage the use of sustainable modes of travel including buses, bicycles and walking. RMS therefore recommends that the planning proposal ensure that it supports and considers, to the greatest extent possible, the aims and objectives of the State Government policies dealing with this matter.

Both public and active transport initiatives will be adopted during the detailed design stages of the development.

Consideration should be given to the identification of appropriate pedestrian and cycle links to/from the development. The applicant should identify suitable infrastructure to ameliorate any safety impacts as a result of the future development of the site.

More details will be provided during the Development Application stage.

6 Conclusion

This report was prepared to outline the traffic and transport impacts associated with Stage 1 of the proposed Reeves Creek residential development in Picton.

The transport assessment has considered the following:

- > Public transport provisions available within the vicinity of the subject development.
- > The optimal configuration and sight distance available for the proposed site access intersection on Menangle Street.
- > Performance of the key intersections in the vicinity of the proposed development for both with and without the proposed development traffic.
- > The concerns outlined by RMS in their letter referenced: STH13/00050

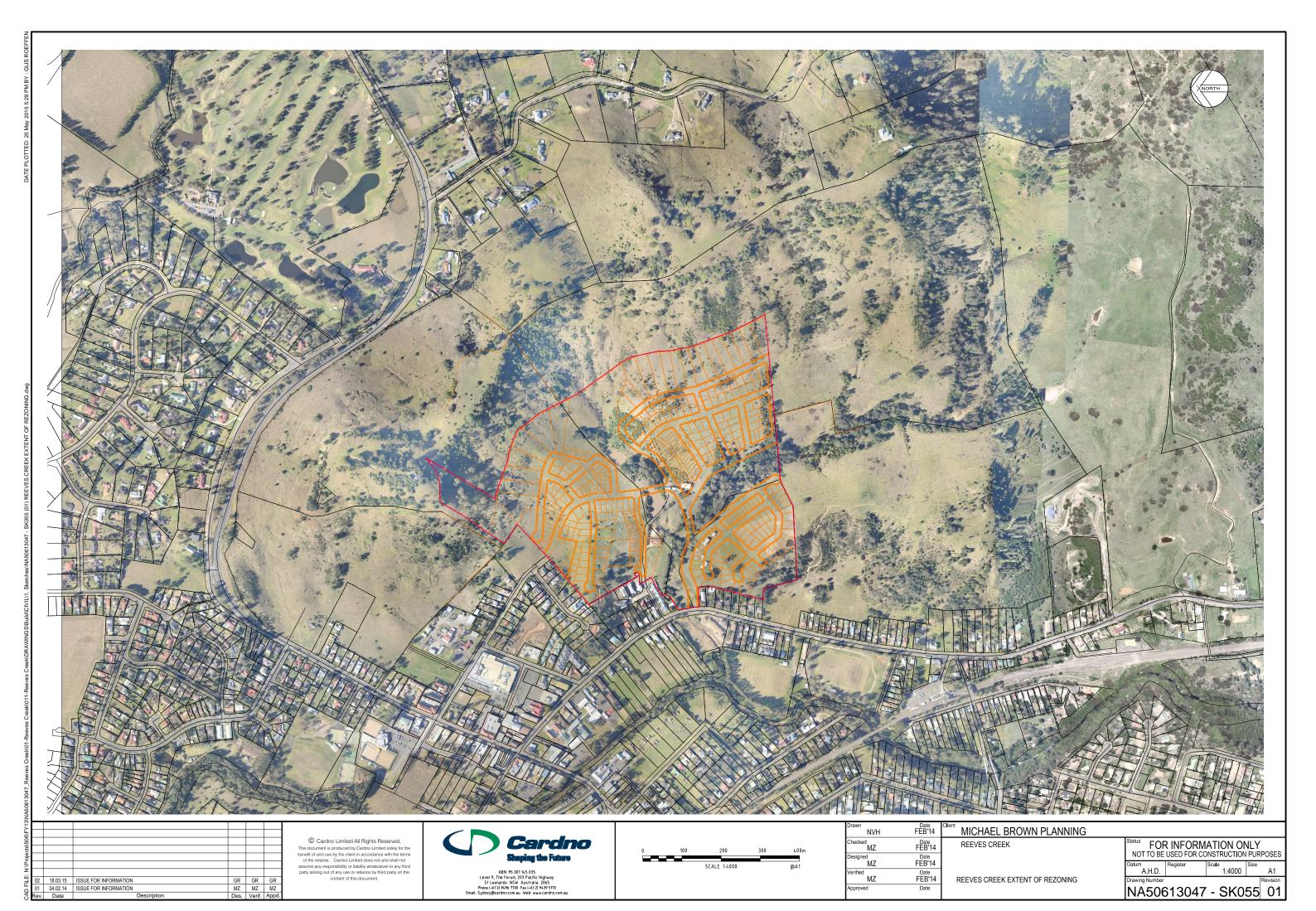
In summary, the findings of the transport assessment are as follows:

- > There are five bus routes which pass nearby the proposed development along Argyle Street with two buss tops located within a 500 metre radius of the proposed development. Also, the Picton Railway Station is located 1km from the subject development.
- > From the analysis presented in this report, with reference to AustRoads Guidelines, it is proposed to upgrade the primary site access to a priority controlled access with auxiliary / deceleration left turn lane and channelised right turn on Menangle Street.
- > The vehicles approaching the site access point can foresee any potential conflicts (with vehicles along Menangle Street) with a clear sight envelop of 123m (with no permanent obstructions present within the sight triangles). As such the Safe Intersection Sight Distance requirement of 123m can be achieved at this location.
- > Based on the Austroads Guide the minimum gap sight distance required is 83m for vehicle exiting the site on to Menangle Street via the site access approach. This requirement can be sufficiently met at the site access intersection with Menangle Street.
- > The 2016 and 2026 baseline traffic volumes were extracted from the Wollondilly Shire TRACKS model.
- > The analysis of the baseline scenarios indicated that the Argyle Street/Menangle Street intersection will need to be upgraded to signalised control by 2016 without the proposed development.
- > The Argyle Street / Cliffe Street / Margaret Street intersection and the Argyle Street/Menangle Street intersection (with proposed upgrades) will perform satisfactorily at the 2016 and 2026 baseline scenario.
- In order to assess the impact of the proposed development on the external road network, the proposed dwelling numbers (400 dwellings) associated with the development were added to the representative zone in the TRACKS model which calculated the trip generation and distributed and assigned trips accordingly.
- > Modelling results for the post-development scenario for Argyle Street/Menangle Street, Argyle Street / Cliffe Street / Margaret Street and Site Access/Menangle Street intersections indicate satisfactory performance.

Traffic Impact Assessment

APPENDIX A
PROPOSED
DEVELOPMENT
MASTERPLAN





Traffic Impact Assessment

APPENDIX B
WOLLONDILLY SHIRE
COUNCIL
CORRESPONDANCE



Hello Dick,

Further to our conversation on Monday 10 March and my email below, could you please provide us with an agreement to use the Wollondilly Shire TRACKS model to undertake the traffic assessment for the Reeves Creek rezoning application located in Picton?

We understand that the layout plans for the proposed signalisation of the Margaret Street / Argyle Street intersection (open 2014) are with your consultants and would appreciate if you are able to provide us with a copy of these plans so that we can use this in our assessment.

As discussed, the methodology of our traffic assessment shall be consistent with that was used for the Stone Quarry rezoning application and therefore the forecast flow, anticipated development trip generation and assignment shall be obtained by utilising the TRACKS model.

In our conversation you confirmed that our traffic assessment for the Reeves Creek development would not need to consider the traffic generated by the Stone Quarry rezoning application as this has not been approved and is not a committed development.

If you would like to discuss any of the above matters further, disagree with any of the above or require any more information, please don't hesitate to contact me. Otherwise we hope to receive the agreement for the TRACKS model shortly and the layout plans of the proposed signalised intersection, so that we can progress with our assessment.

Regards

Devinda Kumarasinghe SENIOR ENGINEER CARDNO



Phone +61 2 9496 7700 Fax +61 2 9439 5170 Direct +61 2 9024 7009 Address Level 9 - The Forum, 203 Pacific Highway, St Leonards, NSW 2065 Australia Postal PO Box 19, St Leonards NSW 1590 Email Devinda.Kumarasinghe@cardno.com.au Web www.cardno.com

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From: Devinda Kumarasinghe (Sydney) **Sent:** Wednesday, 5 March 2014 12:10 PM **To:** 'dick.webb@wollondilly.nsw.gov.au'

Subject: Reeves Creek, Picton Traffic Assessment

Hello Dick,

Further to our conversation yesterday, could you please provide us with an agreement to use the Wollondilly Shire TRACKS model to undertake the traffic assessment for the Reeves Creek rezoning application located in Picton?

We currently have access to the model from previous projects and anticipate that with the agreement we will use the model files currently on our system (more recently our Traffic Engineer, Neill Miller, was in contact with you and obtained the model for use in the traffic assessment for the Stone Quarry Creek rezoning application).

The methodology of our traffic assessment shall be consistent with that was used for the Stone Quarry rezoning application and therefore the forecast flow, anticipated development trip generation and assignment shall be obtained by utilising the TRACKS model.

We understand that the Council is looking to update the TRACKS model, however the timeframe for this update has not been confirmed and would fall outside the delivery timeframe of our project. Therefore, it was agreed to use the TRACKS model that we currently have on our system.

The intersections that we propose to assess are as follows (for the weekday AM and PM peak hour periods):

- Site access / Menangle Street
- Margaret Street / Argyle Street
- Argyle Street / Menangle Street

The design years that will be assessed are 2016 and 2026 for the 'Base' and 'Base plus Development' scenarios.

Please confirm if there are any committed schemes / road upgrades that we should take account of in our assessment. Could you provide us the available concept plans / details for the following proposed upgrades (committed) you noted yesterday:

- Proposed signalisation of the Margaret Street / Argyle Street intersection (open 2014)
- Proposed roundabout at the Argyle Street / Regreme Road intersection (open 2016).

If you would like to discuss any of the above matters further, disagree with anything, or require any more information, please contact me. Otherwise we hope to receive the agreement for the TRACKS model and the proposed intersection plans / details shortly, so that we can progress with our assessment.

Regards

Devinda Kumarasinghe SENIOR ENGINEER CARDNO



Phone +61 2 9496 7700 Fax +61 2 9439 5170 Direct +61 2 9024 7009 Address Level 9 - The Forum, 203 Pacific Highway, St Leonards, NSW 2065 Australia Postal PO Box 19, St Leonards NSW 1590 Email Devinda.Kumarasinghe@cardno.com.au Web www.cardno.com

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Traffic Impact Assessment

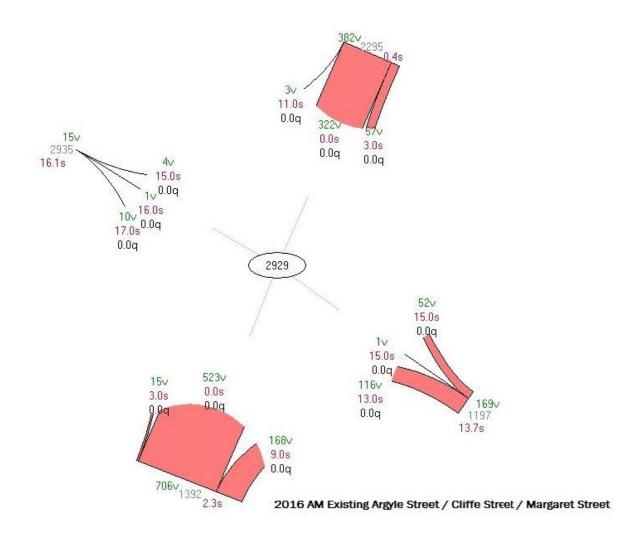
APPENDIX C
TRACKS MODEL
TRAFFIC VOLUMES

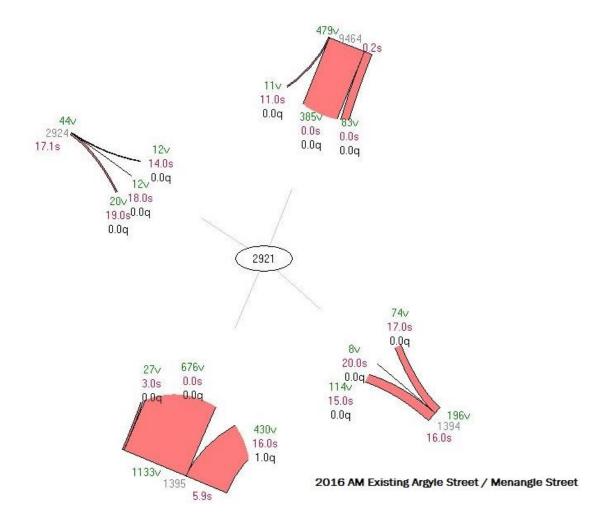


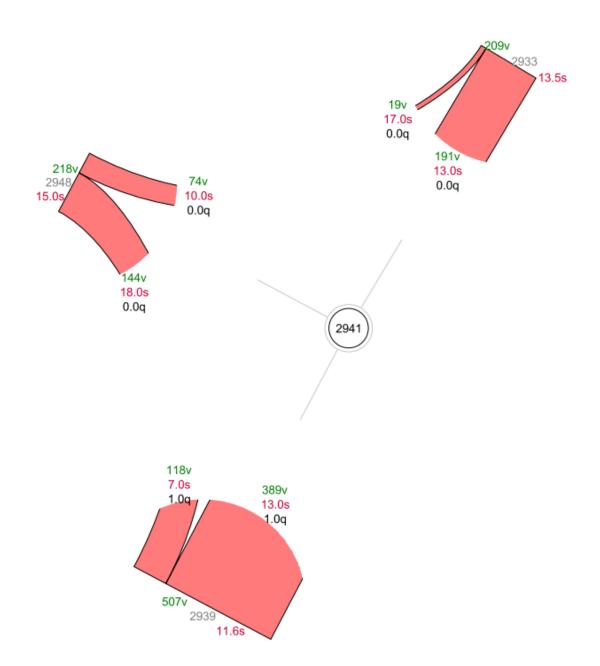
2016 Base

AM Peak

Argyle Street/Cliffe Street/Margaret Street



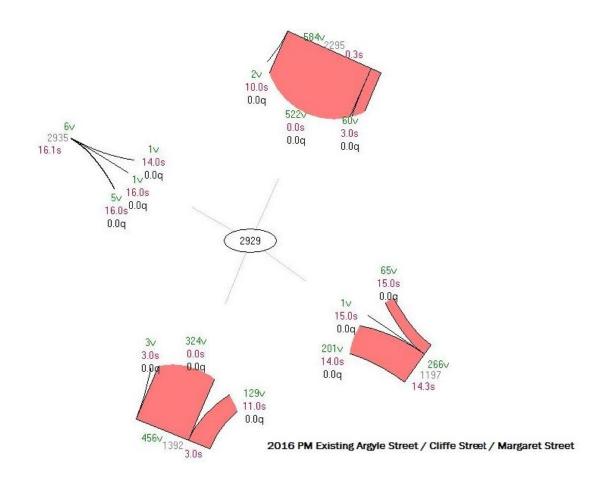


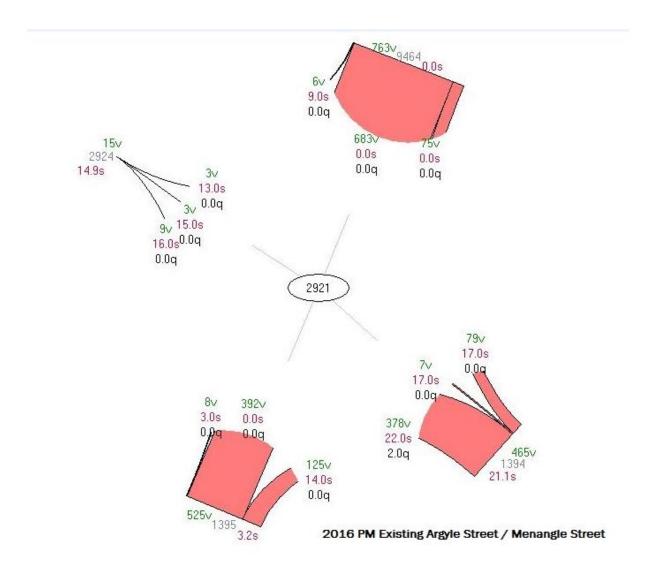


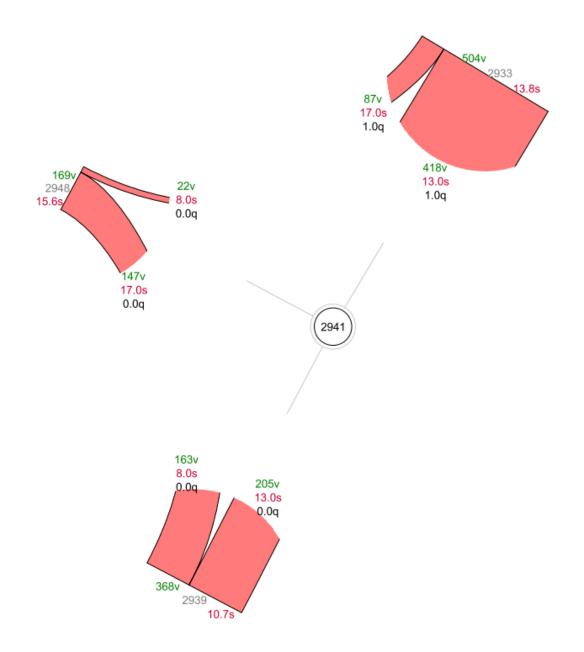
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PM Peak

Argyle Street/Cliffe Street/Margaret Street





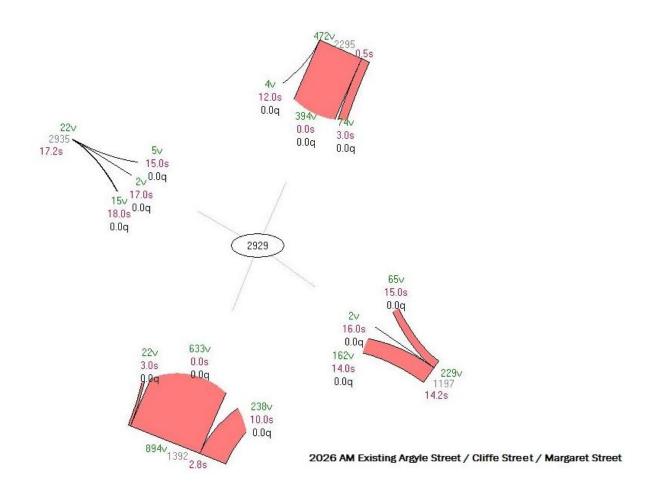


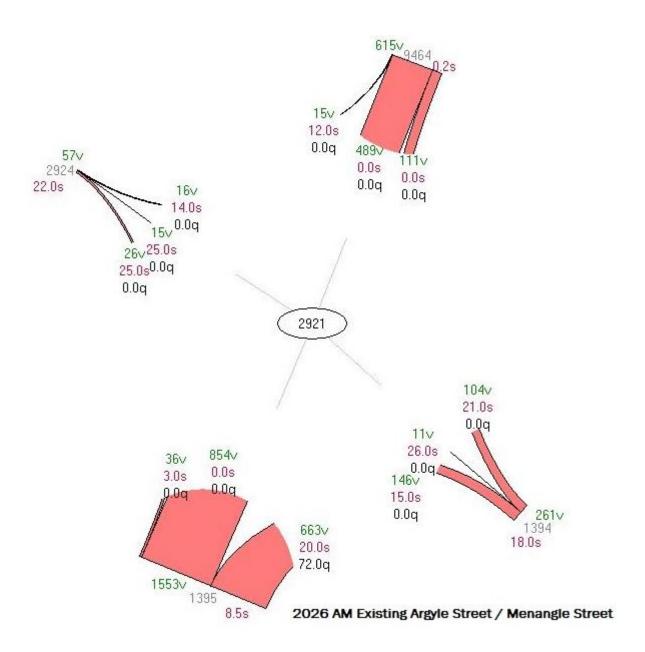
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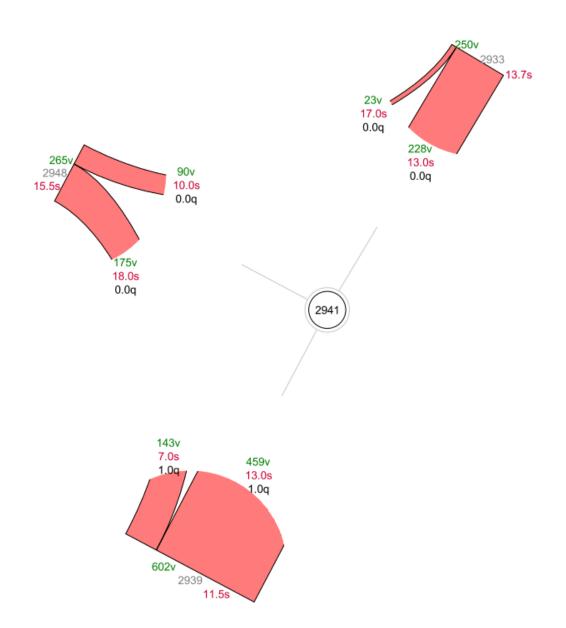
2026 Base

AM Peak

Argyle Street/Cliffe Street/Margaret Street



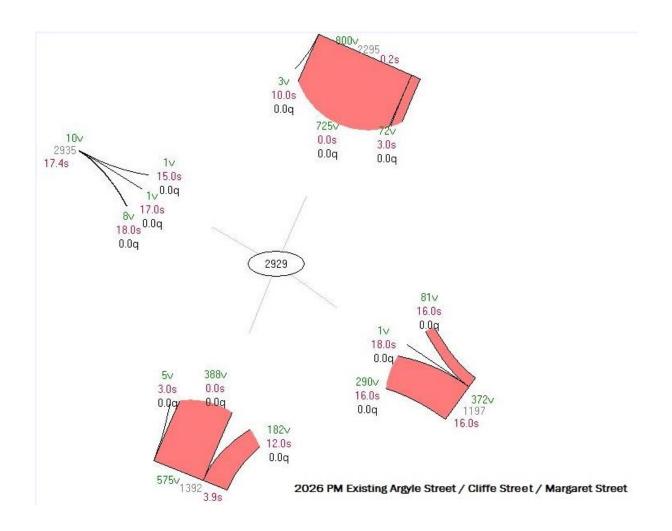


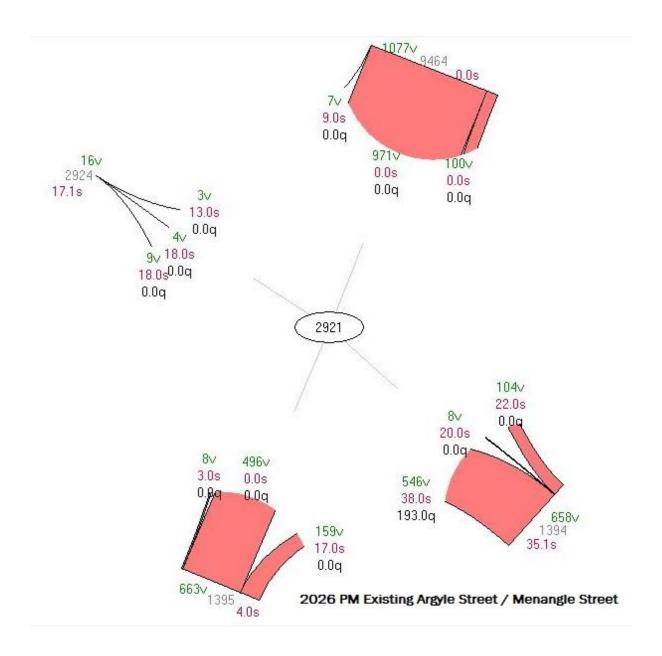


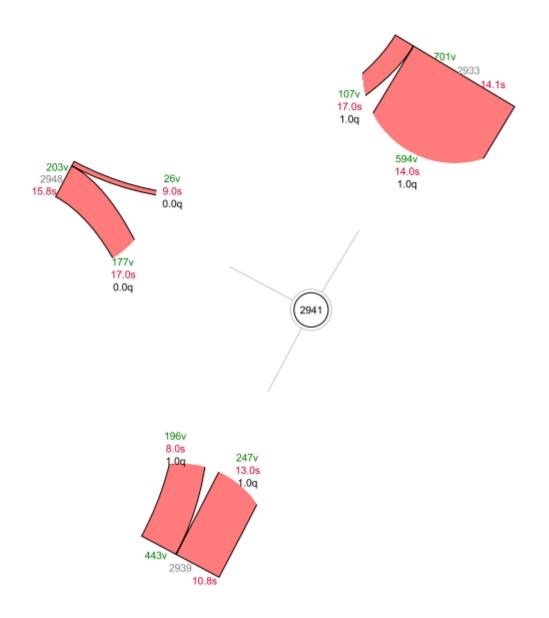
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PM Peak

Argyle Street/Cliffe Street/Margaret Street





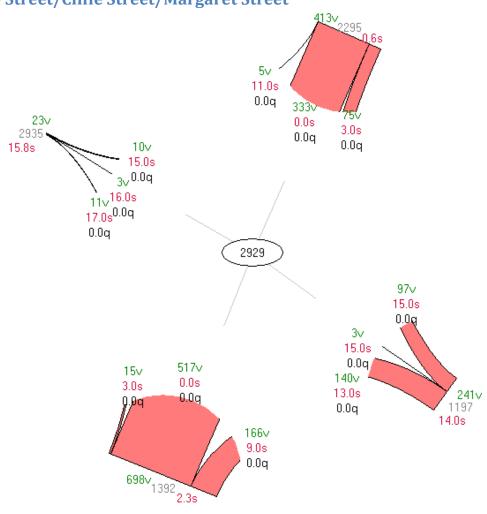


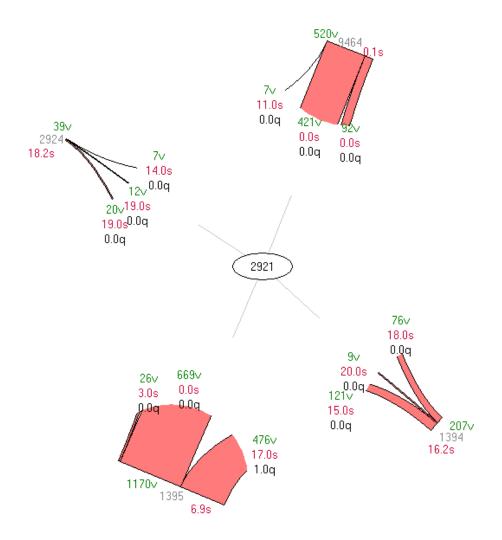
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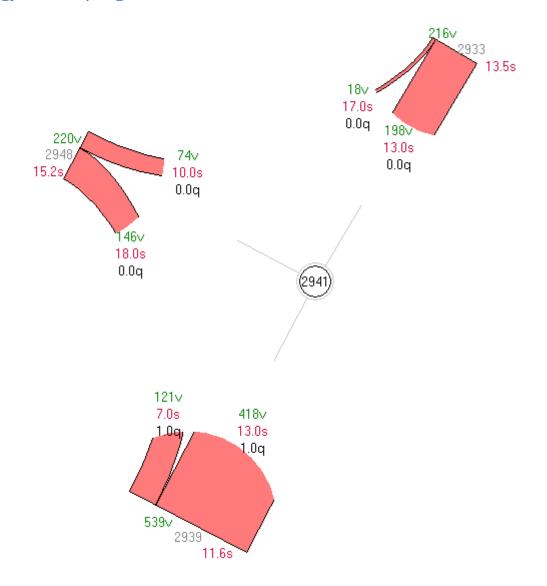
2016 Base + Development

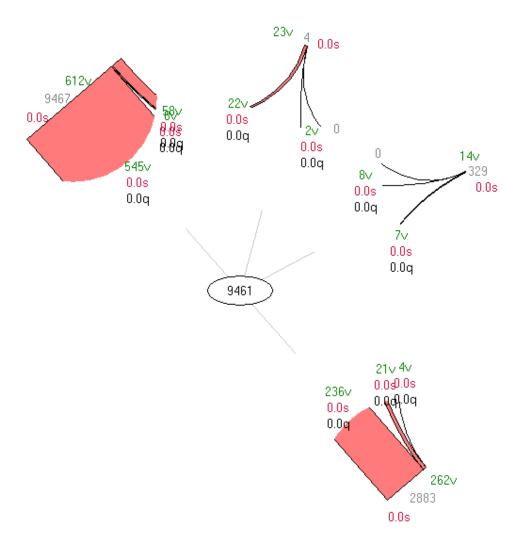
AM Peak





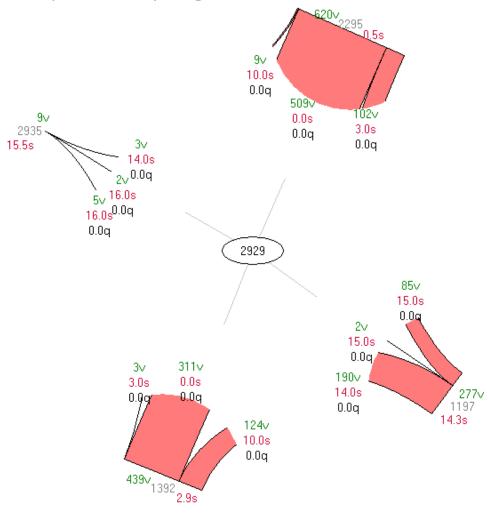


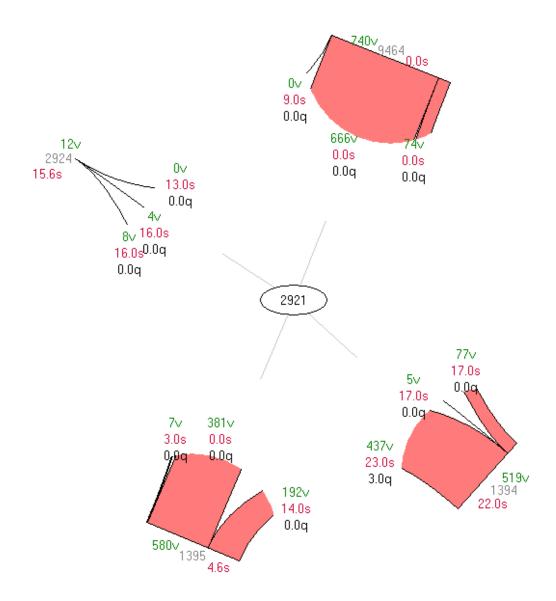


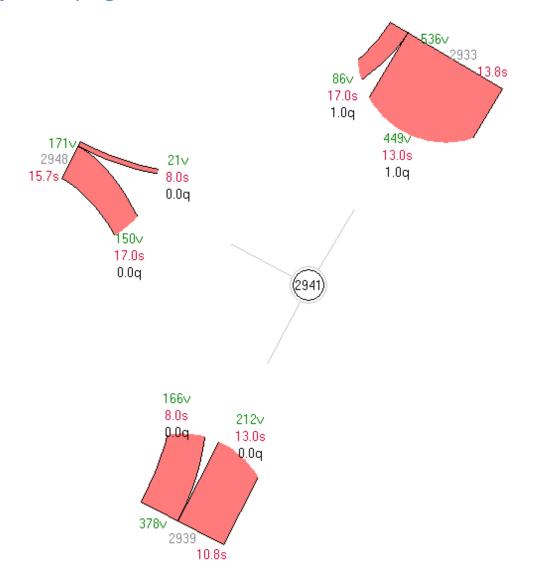


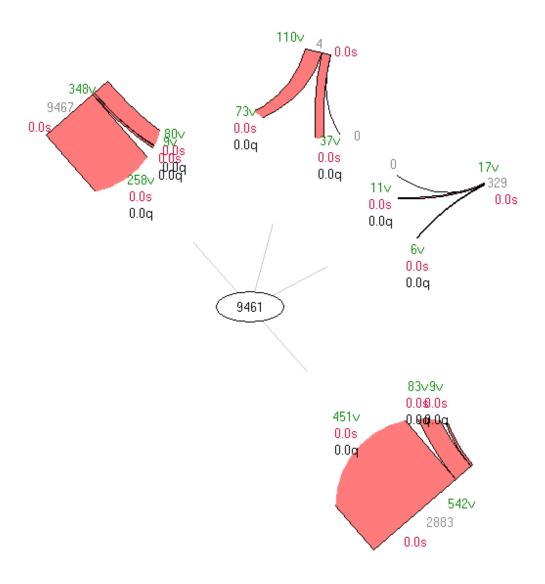
PM Peak

Argyle Street/Cliffe Street/Margaret Street





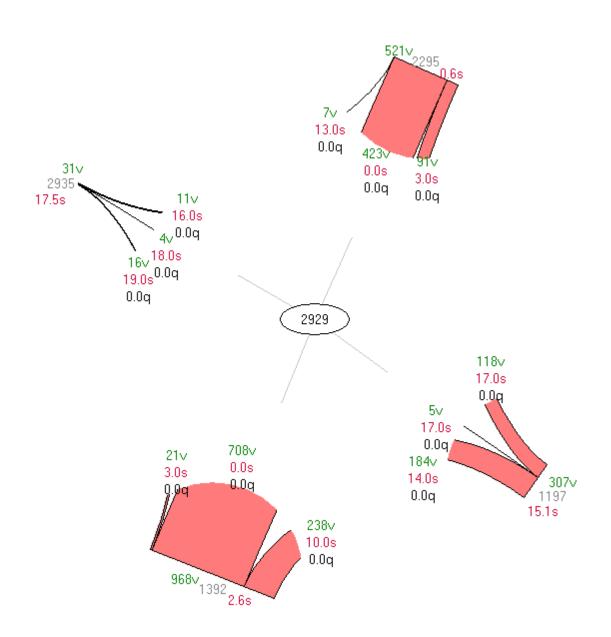


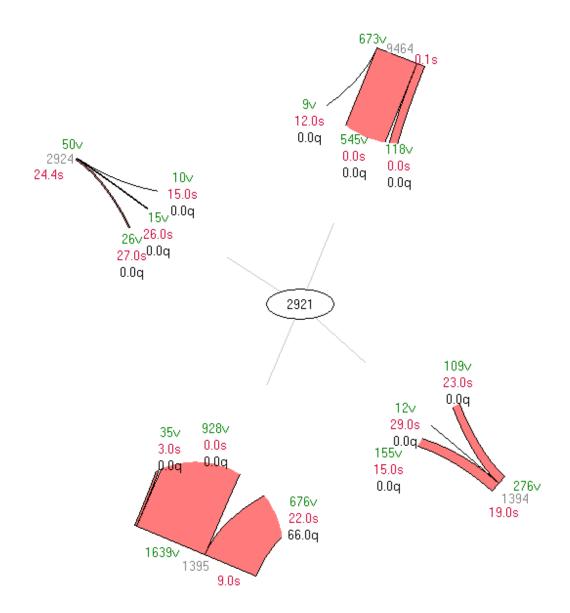


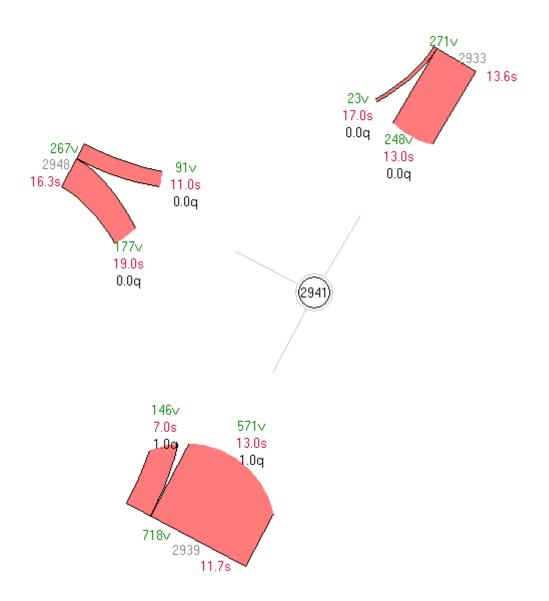
2026 Base + Development

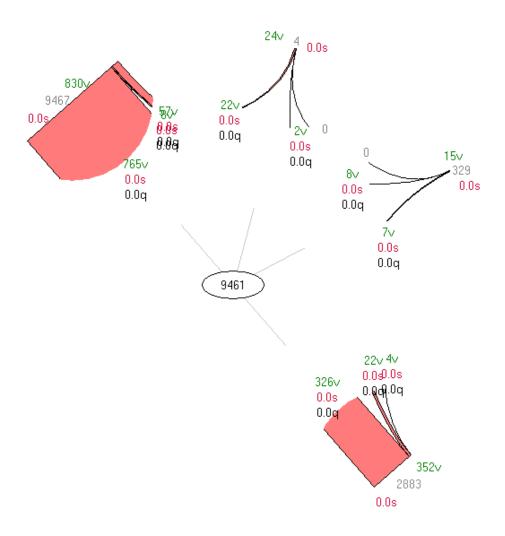
AM Peak

Argyle Street/Cliffe Street/Margaret Street



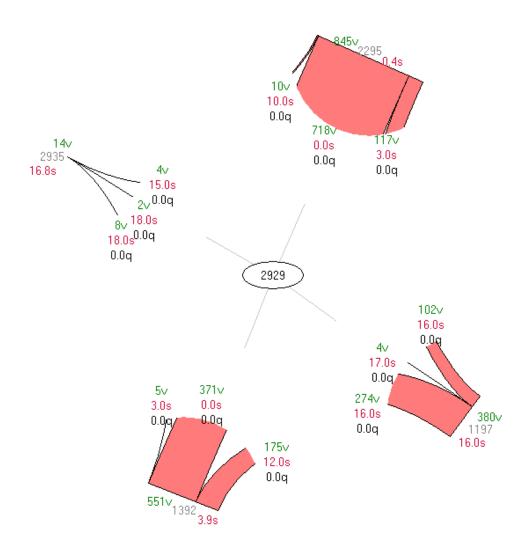


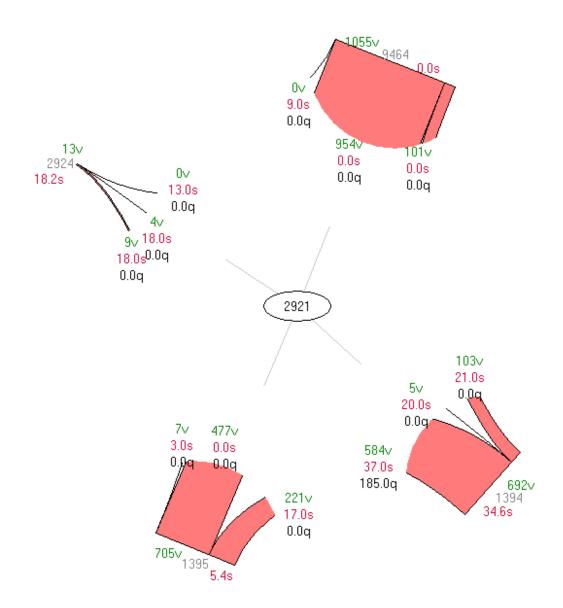


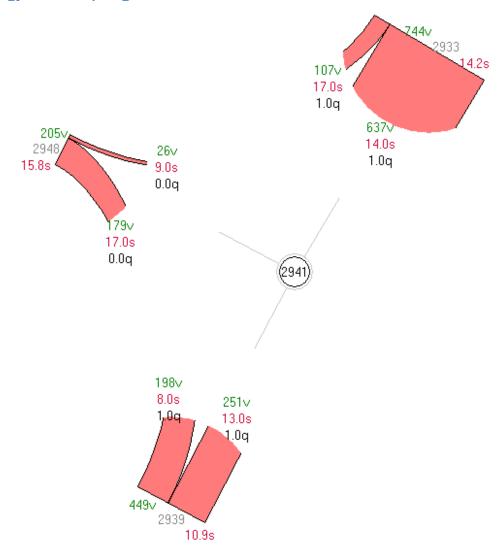


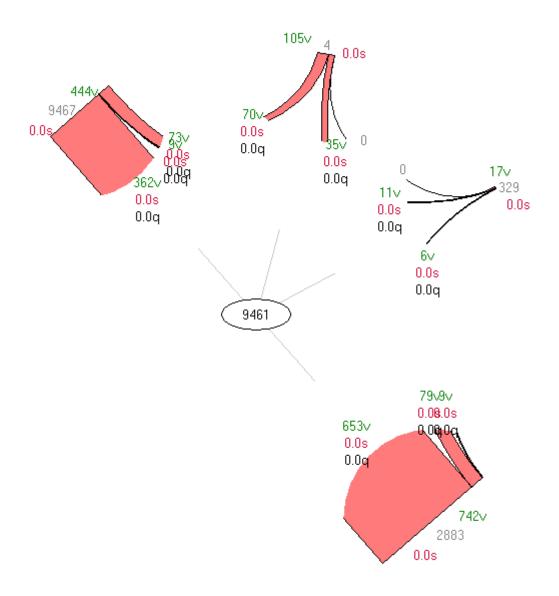
PM Peak

Argyle Street/Cliffe Street/Margaret Street









Traffic Impact Assessment

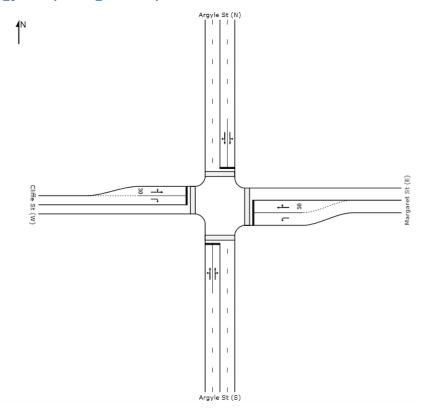
APPENDIX D SIDRA MODEL OUTPUTS



BASELINE SCENARIO - 2016

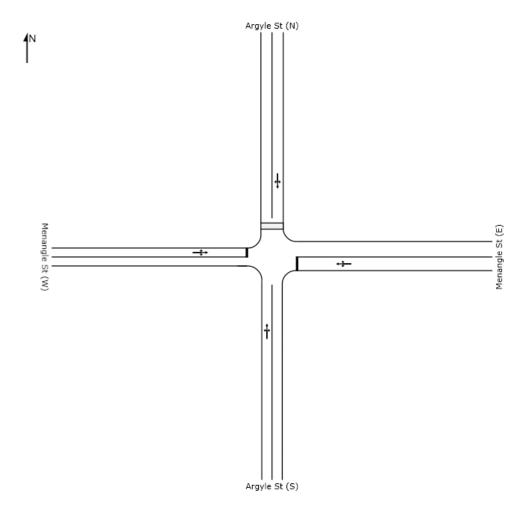
AM PEAK

Argyle St/Margaret St/Cliffe St



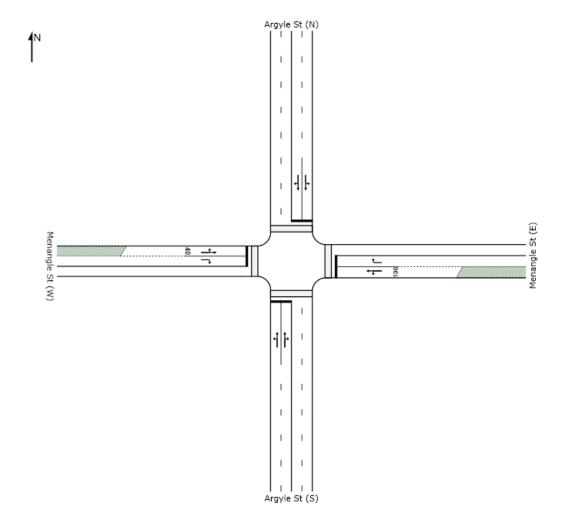
Movement Performance - Vehicles											
Mov ID ODMo Demand Flows Deg. Satn			Average	Level of	95% Back	of Queue	Prop.	Effective	Average		
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: Argyle St (S)											
1	L2	15	2.0	0.449	20.8	LOS B	14.3	104.4	0.62	0.56	35.8
2	T1	523	5.0	0.449	14.8	LOS B	14.3	104.4	0.63	0.58	35.3
3	R2	168	2.0	0.449	24.3	LOS B	8.0	57.5	0.67	0.73	32.1
Approach		706	4.2	0.449	17.2	LOS B	14.3	104.4	0.64	0.62	34.5
East: Margaret St (E)											
4	L2	116	2.0	0.427	54.5	LOS D	5.9	41.7	0.96	0.78	21.6
5	T1	5	2.0	0.209	46.3	LOS D	2.8	19.7	0.93	0.74	22.1
6	R2	52	2.0	0.209	52.8	LOS D	2.8	19.7	0.93	0.74	22.1
Appro	Approach		2.0	0.427	53.8	LOS D	5.9	41.7	0.95	0.77	21.8
North: Argyle St (N)											
7	L2	57	2.0	0.189	18.3	LOS B	5.0	36.4	0.51	0.54	36.8
8	T1	322	5.0	0.189	12.5	LOS A	5.0	36.4	0.52	0.49	36.9
9	R2	5	2.0	0.189	19.3	LOS B	4.9	35.5	0.53	0.46	36.9
Appro	ach	384	4.5	0.189	13.4	LOS A	5.0	36.4	0.52	0.50	36.9
West: Cliffe St (W)											
10	L2	5	2.0	0.038	51.1	LOS D	0.5	3.3	0.89	0.65	9.7
11	T1	5	2.0	0.038	44.7	LOS D	0.5	3.3	0.89	0.65	9.7
12	R2	10	2.0	0.039	51.3	LOS D	0.5	3.3	0.89	0.68	9.4
Approach		20	2.0	0.039	49.6	LOS D	0.5	3.3	0.89	0.66	9.5
All Ve	hicles	1283	4.0	0.449	21.5	LOS B	14.3	104.4	0.65	0.60	32.2

Argyle St/Menangle St

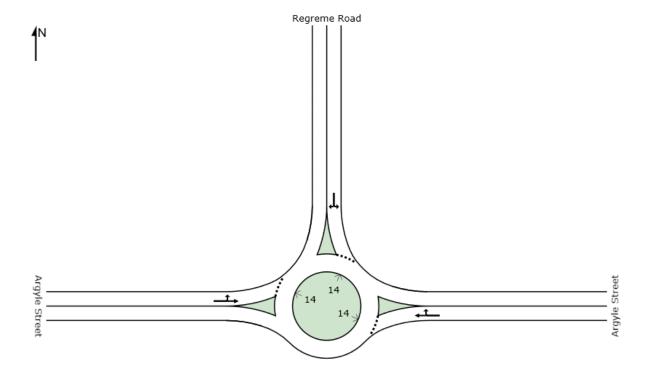


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Movement Performance - Vehicles Mov ID ODMo Demand Flows Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average											
IVIOV IL	O ODMo			eg. Satn	Average	Level of	95% Back		Prop.	Effective	Average
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: Argyle St (S)											
1	L2	27	2.0	0.770	16.5	LOS B	17.8	128.7	1.00	0.57	36.9
2	T1	676	5.0	0.770	10.1	LOS A	17.8	128.7	1.00	0.57	36.9
3	R2	430	2.0	0.770	15.9	LOS B	17.8	128.7	1.00	0.57	36.9
Approach		1133	3.8	0.770	12.5	NA	17.8	128.7	1.00	0.57	36.9
East: Menangle St (E)											
4	L2	114	2.0	1.197	448.0	LOS F	48.6	345.9	1.00	6.37	4.2
5	T1	8	2.0	1.197	447.6	LOS F	48.6	345.9	1.00	6.37	4.2
6	R2	74	2.0	1.197	447.9	LOS F	48.6	345.9	1.00	6.37	4.2
Appro	ach	196	2.0	1.197	448.0	LOS F	48.6	345.9	1.00	6.37	4.2
North: Argyle St (N)											
7	L2	83	2.0	0.260	10.8	LOS A	3.2	23.3	0.76	0.06	41.2
8	T1	385	5.0	0.260	5.2	LOS A	3.2	23.3	0.76	0.06	41.2
9	R2	11	2.0	0.260	11.9	LOS A	3.2	23.3	0.76	0.06	41.2
Appro	ach	479	4.4	0.260	6.3	NA	3.2	23.3	0.76	0.06	41.2
West: Menangle St (W)											
10	L2	12	2.0	0.504	66.8	LOS E	1.6	11.4	0.95	1.08	12.1
11	T1	12	2.0	0.504	65.7	LOS E	1.6	11.4	0.95	1.08	12.1
12	R2	20	2.0	0.504	66.6	LOS E	1.6	11.4	0.95	1.08	12.1
Appro	ach	44	2.0	0.504	66.4	LOS E	1.6	11.4	0.95	1.08	12.1
All Vehicles		1852	3.7	1.197	58.2	NA	48.6	345.9	0.94	1.06	19.9

Argyle St/Menangle St (Proposed Signalised Layout)



Movement Performance - Vehicles											
	O ODMo			Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
1010112	V	Total	HV	Dog. Odin	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: Argyle St (S)									po. 10	1,	
1	L2	27	2.0	0.496	12.8	LOS A	15.1	109.8	0.45	0.43	42.0
2	T1	676	5.0	0.496	6.6	LOS A	15.1	109.8	0.46	0.44	41.7
3	R2	430	2.0	0.496	22.1	LOS B	14.2	101.5	0.71	0.87	31.9
Appro	ach	1133	3.8	0.496	12.6	LOS A	15.1	109.8	0.55	0.60	37.4
East: Menangle St (E)											
4	L2	114	2.0	0.425	51.7	LOS D	6.1	43.2	0.95	0.79	20.3
5	T1	8	2.0	0.425	46.4	LOS D	6.1	43.2	0.95	0.79	20.3
6	R2	74	2.0	0.359	52.8	LOS D	3.7	26.5	0.94	0.77	20.0
Approach		196	2.0	0.425	51.9	LOS D	6.1	43.2	0.95	0.78	20.2
North: Argyle St (N)											
7	L2	83	2.0	0.475	40.3	LOS C	11.4	82.9	0.87	0.77	25.7
8	T1	385	5.0	0.475	36.0	LOS C	11.4	82.9	0.88	0.76	25.6
9	R2	11	2.0	0.475	43.5	LOS D	10.1	73.4	0.89	0.75	25.4
Appro	ach	479	4.4	0.475	36.9	LOS C	11.4	82.9	0.88	0.76	25.6
West: Menangle St (W)											
10	L2	12	2.0	0.083	49.7	LOS D	1.1	7.9	0.89	0.69	14.9
11	T1	12	2.0	0.083	44.6	LOS D	1.1	7.9	0.89	0.69	14.9
12	R2	20	2.0	0.140	57.4	LOS E	1.0	7.3	0.94	0.71	13.5
Approach		44	2.0	0.140	51.8	LOS D	1.1	7.9	0.91	0.70	14.2
All Vel	hicles	1852	3.7	0.496	24.0	LOS B	15.1	109.8	0.69	0.66	30.3



Move	ment Per	formance	- Vehic	cles							
Mov ID	ODMo	Demand	Flows D	Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: A	Argyle Stre	et									
5	T1	191	5.0	0.187	7.0	LOS A	1.1	8.0	0.37	1.10	48.7
6	R2	19	2.0	0.187	11.8	LOS B	1.1	8.0	0.37	1.10	48.7
Approa	ach	210	4.7	0.187	7.4	LOS A	1.1	8.0	0.37	0.55	48.7
North:	Regreme I	Road									
7	L2	74	2.0	0.233	9.5	LOS A	1.3	9.2	0.56	1.49	45.2
9	R2	144	2.0	0.233	13.4	LOS B	1.3	9.2	0.56	1.49	45.2
Approa	ach	218	2.0	0.233	12.1	LOS B	1.3	9.2	0.56	0.74	45.2
West:	Argyle Stre	eet									
10	L2	118	2.0	0.332	7.1	LOS A	2.3	16.9	0.13	0.99	50.3
11	T1	389	5.0	0.332	6.2	LOS A	2.3	16.9	0.13	0.99	50.3
Approa	ach	507	4.3	0.332	6.4	LOS A	2.3	16.9	0.13	0.49	50.3
All Vel	nicles	935	3.9	0.332	8.0	LOS A	2.3	16.9	0.28	0.56	48.6

PM PEAK

Argyle St/Margaret St/Cliffe St

	<i>y</i>	8		<i>'</i>								
Movement Performance - Vehicles Mov ID ODMo Demand Flows Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average												
Mov II	D ODMo	Demand	Flows I	Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average	
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed	
		veh/h	%	v/c	sec		veh	m		per veh	km/h	
South	: Argyle St	(S)										
1	L2	5	2.0	0.367	24.9	LOS B	10.8	79.1	0.66	0.58	33.4	
2	T1	324	5.0	0.367	18.5	LOS B	10.8	79.1	0.66	0.58	33.4	
3	R2	129	2.0	0.452	33.8	LOS C	5.3	37.7	0.79	0.79	27.6	
Appro	ach	458	4.1	0.452	22.9	LOS B	10.8	79.1	0.70	0.64	31.5	
East:	Margaret St	(E)										
4	L2	201	2.0	0.462	47.1	LOS D	9.5	67.5	0.92	0.81	23.4	
5	T1	5	2.0	0.160	37.6	LOS C	3.0	21.7	0.85	0.75	24.3	
6	R2	65	2.0	0.160	44.0	LOS D	3.0	21.7	0.85	0.75	24.3	
Appro	ach	271	2.0	0.462	46.2	LOS D	9.5	67.5	0.90	0.79	23.6	
North:	: Argyle St (N)										
7	L2	60	2.0	0.332	24.5	LOS B	9.6	69.9	0.65	0.62	33.2	
8	T1	522	5.0	0.332	18.4	LOS B	9.6	69.9	0.65	0.59	33.2	
9	R2	5	2.0	0.332	25.1	LOS B	9.5	69.0	0.66	0.57	33.3	
Appro	ach	587	4.7	0.332	19.1	LOS B	9.6	69.9	0.65	0.60	33.2	
West:	Cliffe St (W	/)										
10	L2	5	2.0	0.038	51.1	LOS D	0.5	3.3	0.89	0.65	9.7	
11	T1	5	2.0	0.038	44.7	LOS D	0.5	3.3	0.89	0.65	9.7	
12	R2	5	2.0	0.020	51.0	LOS D	0.2	1.7	0.89	0.65	9.4	
Appro	ach	15	2.0	0.038	48.9	LOS D	0.5	3.3	0.89	0.65	9.6	
All Ve	hicles	1331	3.9	0.462	26.3	LOS B	10.8	79.1	0.72	0.65	29.9	

Argyle St/Menangle St

Movement Performance - Vehicles Mov ID ODMo Demand Flows Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average												
	O ODMo v				Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed	
		veh/h	%	v/c	sec		veh	m		per veh	km/h	
South	: Argyle St	(S)										
1	L2	8	2.0	0.389	14.0	LOS A	5.0	36.6	0.95	0.27	39.2	
2	T1	392	5.0	0.389	7.6	LOS A	5.0	36.6	0.95	0.27	39.2	
3	R2	125	2.0	0.389	14.1	LOS A	5.0	36.6	0.95	0.27	39.2	
Appro	ach	525	4.2	0.389	9.2	NA	5.0	36.6	0.95	0.27	39.2	
East:	Menangle S	St (E)										
4	L2	378	2.0	1.306	584.4	LOS F	144.8	1030.7	1.00	10.28	3.3	
5	T1	7	2.0	1.306	584.0	LOS F	144.8	1030.7	1.00	10.28	3.3	
6	R2	79	2.0	1.306	584.3	LOS F	144.8	1030.7	1.00	10.28	3.3	
Appro	ach	464	2.0	1.306	584.4	LOS F	144.8	1030.7	1.00	10.28	3.3	
North:	Argyle St (N)										
7	L2	75	2.0	0.402	9.4	LOS A	4.8	35.3	0.66	0.04	42.5	
8	T1	683	5.0	0.402	2.9	LOS A	4.8	35.3	0.66	0.04	42.5	
9	R2	6	2.0	0.402	9.7	LOS A	4.8	35.3	0.66	0.04	42.5	
Appro	ach	764	4.7	0.402	3.6	NA	4.8	35.3	0.66	0.04	42.5	
West:	Menangle :	St (W)										
10	L2	5	2.0	0.114	29.3	LOS C	0.3	2.4	0.83	0.97	21.7	
11	T1	5	2.0	0.114	28.9	LOS C	0.3	2.4	0.83	0.97	21.7	
12	R2	9	2.0	0.114	29.2	LOS C	0.3	2.4	0.83	0.97	21.7	
Appro	ach	19	2.0	0.114	29.2	LOS C	0.3	2.4	0.83	0.97	21.7	
All Ve	hicles	1772	3.8	1.306	157.6	NA	144.8	1030.7	0.83	2.80	10.3	

Argyle St/Menangle St (Proposed Signalised Layout)

Movement Performance - Vehicles Mov ID ODMo Demand Flows Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average												
Mov II	O ODMo	Demand	Flows D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average	
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed	
		veh/h	%	v/c	sec		veh	m		per veh	km/h	
South	: Argyle St	(S)										
1	L2	8	2.0	0.407	23.6	LOS B	12.9	94.0	0.65	0.58	34.1	
2	T1	392	5.0	0.407	17.3	LOS B	12.9	94.0	0.66	0.58	34.1	
3	R2	125	2.0	0.407	36.4	LOS C	5.6	40.0	0.87	0.80	25.9	
Appro	ach	525	4.2	0.407	21.9	LOS B	12.9	94.0	0.71	0.63	31.7	
East:	Menangle S	St (E)										
4	L2	378	2.0	0.558	34.7	LOS C	16.3	116.3	0.85	0.82	24.3	
5	T1	7	2.0	0.558	29.4	LOS C	16.3	116.3	0.85	0.82	24.3	
6	R2	79	2.0	0.167	30.4	LOS C	2.8	20.3	0.70	0.74	25.6	
Appro	ach	464	2.0	0.558	33.8	LOS C	16.3	116.3	0.82	0.81	24.5	
North:	Argyle St	(N)										
7	L2	75	2.0	0.543	34.1	LOS C	16.2	117.9	0.83	0.75	28.2	
8	T1	683	5.0	0.543	28.8	LOS C	16.2	117.9	0.84	0.74	28.3	
9	R2	6	2.0	0.543	35.6	LOS C	15.8	115.3	0.84	0.73	28.3	
Appro	ach	764	4.7	0.543	29.4	LOS C	16.2	117.9	0.84	0.74	28.2	
West:	Menangle	St (W)										
10	L2	5	2.0	0.014	29.3	LOS C	0.3	2.4	0.65	0.60	21.3	
11	T1	5	2.0	0.014	24.1	LOS B	0.3	2.4	0.65	0.60	21.3	
12	R2	9	2.0	0.043	44.7	LOS D	0.4	2.8	0.82	0.69	16.1	
Appro	ach	19	2.0	0.043	35.3	LOS C	0.4	2.8	0.73	0.64	18.4	
All Ve	hicles	1772	3.8	0.558	28.4	LOS B	16.3	117.9	0.79	0.73	27.9	

Move	Movement Performance - Vehicles Mov ID ODMo Demand Flows Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average													
Mov II	ODMo	Demand	Flows	Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average			
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed			
		veh/h	%	v/c	sec		veh	m		per veh	km/h			
East: A	Argyle Stree	et												
5	T1	418	5.0	0.424	7.2	LOS A	3.1	22.5	0.45	1.17	47.9			
6	R2	87	2.0	0.424	12.0	LOS B	3.1	22.5	0.45	1.17	47.9			
Approa	ach	505	4.5	0.424	8.0	LOS A	3.1	22.5	0.45	0.58	47.9			
North: Regreme		Road												
7	L2	22	2.0	0.157	8.2	LOS A	8.0	6.0	0.41	1.36	45.4			
9	R2	147	2.0	0.157	12.1	LOS B	8.0	6.0	0.41	1.36	45.4			
Approa	ach	169	2.0	0.157	11.6	LOS B	8.0	6.0	0.41	0.68	45.4			
West:	Argyle Stre	et												
10	L2	163	2.0	0.287	7.5	LOS A	1.9	13.5	0.30	1.05	49.1			
11	T1	205	5.0	0.287	6.6	LOS A	1.9	13.5	0.30	1.05	49.1			
Approa	ach	368	3.7	0.287	7.0	LOS A	1.9	13.5	0.30	0.53	49.1			
All Vel	nicles	1042	3.8	0.424	8.2	LOS A	3.1	22.5	0.39	0.58	47.9			

BASELINE SCENARIO - 2026

AM PEAK

Argyle St/Margaret St/Cliffe St

Maria	mant Dan	f	Vahi	alaa							
		formance									
Mov II	ODMo			Deg. Satn	Average	Level of	95% Back		Prop.	Effective	Average
	V	Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Argyle St ((S)									
1	L2	22	2.0	0.625	23.0	LOS B	23.1	168.5	0.72	0.66	34.3
2	T1	633	5.0	0.625	16.6	LOS B	23.1	168.5	0.72	0.66	34.3
3	R2	238	2.0	0.625	28.9	LOS C	9.6	68.6	0.79	0.82	29.5
Approa	ach	893	4.1	0.625	20.1	LOS B	23.1	168.5	0.74	0.70	32.9
East: N	Margaret St	(E)									
4	L2	162	2.0	0.596	55.9	LOS D	8.4	59.9	0.99	0.81	21.3
5	T1	5	2.0	0.257	46.7	LOS D	3.4	24.4	0.93	0.76	21.9
6	R2	65	2.0	0.257	53.2	LOS D	3.4	24.4	0.93	0.76	21.9
Approa	ach	232	2.0	0.596	54.9	LOS D	8.4	59.9	0.97	0.79	21.5
North:	Argyle St (N)									
7	L2	74	2.0	0.236	18.7	LOS B	6.5	46.9	0.53	0.56	36.5
8	T1	394	5.0	0.236	13.4	LOS A	6.5	46.9	0.55	0.52	36.2
9	R2	5	2.0	0.236	20.8	LOS B	6.3	46.0	0.57	0.49	35.9
Approa	ach	473	4.5	0.236	14.3	LOS A	6.5	46.9	0.55	0.53	36.2
West:	Cliffe St (W	')									
10	L2	5	2.0	0.038	51.1	LOS D	0.5	3.3	0.89	0.65	9.7
11	T1	5	2.0	0.038	44.7	LOS D	0.5	3.3	0.89	0.65	9.7
12	R2	15	2.0	0.059	51.6	LOS D	0.7	5.0	0.90	0.69	9.3
Approa	ach	25	2.0	0.059	50.1	LOS D	0.7	5.0	0.90	0.68	9.5
All Vel		1623	3.9	0.625	23.8	LOS B	23.1	168.5	0.72	0.66	31.0

Move	Movement Performance - Vehicles Mov ID ODMo Demand Flows Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average													
Mov II	D ODMo	Demand	Flows D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average			
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed			
		veh/h	%	v/c	sec		veh	m		per veh	km/h			
South	: Argyle St (S)												
1	L2	36	2.0	0.633	13.5	LOS A	22.6	164.8	0.52	0.50	41.1			
2	T1	854	5.0	0.633	7.1	LOS A	22.6	164.8	0.52	0.50	41.1			
3	R2	663	2.0	0.771	32.3	LOS C	23.7	168.5	0.90	1.03	27.4			
Appro	ach	1553	3.6	0.771	18.0	LOS B	23.7	168.5	0.68	0.73	34.0			
East: Menangle S		t (E)												
4	L2	146	2.0	0.581	53.9	LOS D	8.1	57.7	0.98	0.80	19.8			
5	T1	11	2.0	0.581	48.6	LOS D	8.1	57.7	0.98	0.80	19.8			
6	R2	104	2.0	0.545	55.4	LOS D	5.5	38.8	0.98	0.79	19.5			
Appro	ach	261	2.0	0.581	54.3	LOS D	8.1	57.7	0.98	0.80	19.7			
North:	: Argyle St (N	۷)												
7	L2	111	2.0	0.675	43.6	LOS D	17.1	123.6	0.95	0.83	24.7			
8	T1	489	5.0	0.675	41.1	LOS C	17.1	123.6	0.96	0.83	24.0			
9	R2	15	2.0	0.675	50.6	LOS D	13.1	95.7	0.98	0.83	23.3			
Appro	ach	615	4.4	0.675	41.8	LOS C	17.1	123.6	0.96	0.83	24.1			
West:	Menangle S	St (W)												
10	L2	16	2.0	0.114	51.0	LOS D	1.5	10.4	0.90	0.70	14.6			
11	T1	15	2.0	0.114	45.9	LOS D	1.5	10.4	0.90	0.70	14.6			

12	R2	26	2.0	0.234	61.6	LOS E	1.4	10.0	0.98	0.72	12.8
Appro	ach	57	2.0	0.234	54.5	LOS D	1.5	10.4	0.94	0.71	13.7
All Vel	hicles	2486	3.6	0.771	28.5	LOS C	23.7	168.5	0.79	0.76	28.3

Move	Movement Performance - Vehicles Mov ID ODMo Demand Flows Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average													
Mov ID	ODMo	Demand	Flows D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average			
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed			
		veh/h	%	v/c	sec		veh	m		per veh	km/h			
East: A	Argyle Stree	et												
5	T1	228	5.0	0.231	7.2	LOS A	1.4	10.4	0.43	1.15	48.3			
6	R2	23	2.0	0.231	12.0	LOS B	1.4	10.4	0.43	1.15	48.3			
Approa	ach	251	4.7	0.231	7.6	LOS A	1.4	10.4	0.43	0.57	48.3			
North:	Regreme F	Road												
7	L2	90	2.0	0.300	10.1	LOS B	1.8	12.6	0.63	1.57	44.6			
9	R2	175	2.0	0.300	14.1	LOS B	1.8	12.6	0.63	1.57	44.6			
Approa	ach	265	2.0	0.300	12.8	LOS B	1.8	12.6	0.63	0.79	44.6			
West:	Argyle Stre	et												
10	L2	143	2.0	0.396	7.1	LOS A	3.1	22.3	0.16	0.98	50.1			
11	T1	459	5.0	0.396	6.3	LOS A	3.1	22.3	0.16	0.98	50.1			
Approa	ach	602	4.3	0.396	6.5	LOS A	3.1	22.3	0.16	0.49	50.1			
All Vel	nicles	1118	3.8	0.396	8.2	LOS A	3.1	22.3	0.33	0.58	48.3			

PM PEAK

Argyle St/Margaret St/Cliffe St

Move	Movement Performance - Vehicles Mov ID ODMo Demand Flows Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average												
					Average	Level of	95% Back	of Queue	Prop.	Effective	Average		
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed		
		veh/h	%	v/c	sec		veh	m		per veh	km/h		
South:	Argyle St (S)											
1	L2	5	2.0	0.437	27.4	LOS B	14.7	107.4	0.69	0.61	32.1		
2	T1	388	5.0	0.437	21.0	LOS B	14.7	107.4	0.69	0.61	32.1		
3	R2	182	2.0	0.890	78.5	LOS F	14.3	101.5	1.00	1.14	17.3		
Appro	ach	575	4.0	0.890	39.3	LOS C	14.7	107.4	0.79	0.78	25.2		
East: I	Margaret St	(E)											
4	L2	290	2.0	0.880	64.2	LOS E	18.1	128.9	0.94	0.98	19.6		
5	T1	5	2.0	0.178	38.7	LOS C	4.0	28.3	0.83	0.75	24.0		
6	R2	81	2.0	0.178	45.1	LOS D	4.0	28.3	0.83	0.75	24.0		
Appro		376	2.0	0.880	59.8	LOS E	18.1	128.9	0.92	0.93	20.5		
North:	Argyle St (N)											
7	L2	72	2.0	0.452	27.6	LOS B	15.4	111.9	0.70	0.66	31.6		
8	T1	725	5.0	0.452	21.5	LOS B	15.4	111.9	0.71	0.64	31.6		
9	R2	5	2.0	0.452	28.2	LOS B	15.1	110.2	0.71	0.63	31.6		
Appro	ach	802	4.7	0.452	22.1	LOS B	15.4	111.9	0.71	0.64	31.6		
West:	Cliffe St (W	')											
10	L2	5	2.0	0.042	56.5	LOS D	0.5	3.7	0.91	0.65	8.9		
11	T1	5	2.0	0.042	50.0	LOS D	0.5	3.7	0.91	0.65	8.9		
12	R2	8	2.0	0.034	56.5	LOS E	0.4	2.9	0.90	0.67	8.7		
Appro	ach	18	2.0	0.042	54.7	LOS D	0.5	3.7	0.91	0.66	8.8		
All Vel	nicles	1771	3.9	0.890	36.0	LOS C	18.1	128.9	0.78	0.75	26.2		

Move	ement Per	formance	- Vehic	les							
	D ODMo			eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Argyle St	(S)									
1	L2	8	2.0	0.474	21.6	LOS B	15.9	115.9	0.64	0.58	35.3
2	T1	496	5.0	0.474	15.2	LOS B	15.9	115.9	0.64	0.58	35.3
3	R2	159	2.0	0.630	46.7	LOS D	8.3	58.9	1.00	0.91	22.8
Appro	ach	663	4.2	0.630	22.8	LOS B	15.9	115.9	0.73	0.66	31.3
East:	Menangle S	St (E)									
4	L2	546	2.0	0.669	30.5	LOS C	23.2	165.1	0.84	0.84	25.6
5	T1	8	2.0	0.669	25.2	LOS B	23.2	165.1	0.84	0.84	25.6
6	R2	104	2.0	0.248	34.9	LOS C	4.1	29.4	0.77	0.76	24.2
Appro	ach	658	2.0	0.669	31.1	LOS C	23.2	165.1	0.83	0.83	25.3
North	: Argyle St ((N)									
7	L2	100	2.0	0.668	32.1	LOS C	23.3	169.1	0.86	0.79	29.0
8	T1	971	5.0	0.668	26.8	LOS B	23.3	169.1	0.86	0.78	29.1
9	R2	7	2.0	0.668	33.5	LOS C	22.7	165.8	0.86	0.77	29.1
Appro	ach	1078	4.7	0.668	27.3	LOS B	23.3	169.1	0.86	0.78	29.1
West	Menangle	St (W)									
10	L2	5	2.0	0.016	32.8	LOS C	0.4	2.5	0.70	0.61	19.8
11	T1	5	2.0	0.016	27.7	LOS B	0.4	2.5	0.70	0.61	19.8
12	R2	9	2.0	0.050	44.2	LOS D	0.4	2.8	0.82	0.69	16.2
Appro	ach	19	2.0	0.050	36.9	LOS C	0.4	2.8	0.75	0.65	17.9
All Ve	hicles	2418	3.8	0.669	27.2	LOS B	23.3	169.1	0.81	0.76	28.4

Movement Performance - Vehicles Mov ID ODMo Demand Flows Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average												
Mov ID	ODMo	Demand	Flows	Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average	
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed	
		veh/h	%	v/c	sec		veh	m		per veh	km/h	
East: A	rgyle Stree	et										
5	T1	594	5.0	0.602	7.8	LOS A	5.5	39.9	0.61	1.25	47.2	
6	R2	107	2.0	0.602	12.6	LOS B	5.5	39.9	0.61	1.25	47.2	
Approach		701	4.5	0.602	8.5	LOS A	5.5	39.9	0.61	0.62	47.2	
North: Regreme R		Road										
7	L2	26	2.0	0.197	8.5	LOS A	1.1	7.9	0.47	1.39	45.1	
9	R2	177	2.0	0.197	12.4	LOS B	1.1	7.9	0.47	1.39	45.1	
Approa	ach	203	2.0	0.197	11.9	LOS B	1.1	7.9	0.47	0.70	45.1	
West:	Argyle Stre	et										
10	L2	196	2.0	0.355	7.7	LOS A	2.6	18.5	0.37	1.08	48.7	
11	T1	247	5.0	0.355	6.8	LOS A	2.6	18.5	0.37	1.08	48.7	
Approa	ach	443	3.7	0.355	7.2	LOS A	2.6	18.5	0.37	0.54	48.7	
All Veh	nicles	1347	3.9	0.602	8.6	LOS A	5.5	39.9	0.51	0.61	47.3	

BASE plus DEVELOPMENT SCENARIO – 2016

AM PEAK

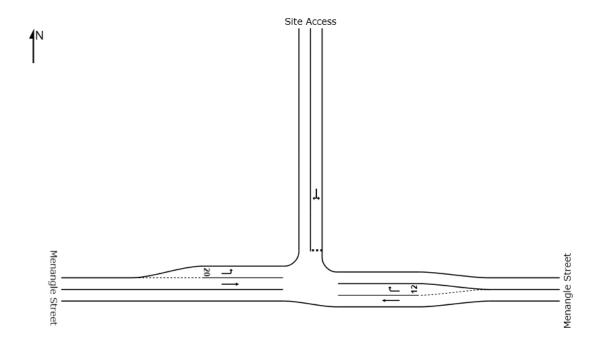
Argyle St/Margaret St/Cliffe St

Movement Performance - Vehicles Mov ID ODMo Demand Flows Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average													
Mov II	D ODMo	Demand	Flows D	Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average		
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed		
		veh/h	%	v/c	sec		veh	m		per veh	km/h		
South	: Argyle St	(S)											
1	L2	15	2.0	0.467	22.1	LOS B	15.1	110.0	0.65	0.58	35.0		
2	T1	517	5.0	0.467	16.2	LOS B	15.1	110.0	0.65	0.60	34.5		
3	R2	166	2.0	0.467	26.4	LOS B	8.1	57.8	0.71	0.75	31.1		
Appro	ach	698	4.2	0.467	18.7	LOS B	15.1	110.0	0.67	0.64	33.6		
East:	Margaret S	t (E)											
4	L2	140	2.0	0.455	52.9	LOS D	7.0	49.7	0.96	0.79	22.0		
5	T1	5	2.0	0.330	45.4	LOS D	5.0	35.4	0.93	0.77	22.2		
6	R2	97	2.0	0.330	51.9	LOS D	5.0	35.4	0.93	0.77	22.2		
Appro	ach	242	2.0	0.455	52.4	LOS D	7.0	49.7	0.95	0.79	22.1		
North:	Argyle St (N)											
7	L2	75	2.0	0.210	19.0	LOS B	5.6	40.4	0.53	0.57	36.1		
8	T1	333	5.0	0.210	13.5	LOS A	5.6	40.4	0.54	0.52	36.1		
9	R2	5	2.0	0.210	20.5	LOS B	5.5	40.1	0.56	0.48	36.1		
Appro	ach	413	4.4	0.210	14.6	LOS B	5.6	40.4	0.54	0.53	36.1		
West:	Cliffe St (V	/)											
10	L2	10	2.0	0.058	51.4	LOS D	0.7	5.0	0.90	0.68	9.6		
11	T1	5	2.0	0.058	45.0	LOS D	0.7	5.0	0.90	0.68	9.6		
12	R2	11	2.0	0.043	51.4	LOS D	0.5	3.7	0.90	0.68	9.4		
Appro	ach	26	2.0	0.058	50.2	LOS D	0.7	5.0	0.90	0.68	9.5		
All Ve	hicles	1379	3.8	0.467	24.0	LOS B	15.1	110.0	0.68	0.63	31.0		

Move	Movement Performance - Vehicles Mov ID ODMo Demand Flows Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average													
Mov II	D ODMo v	Demand Total	Flows D HV	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed			
		veh/h	%	v/c	sec		veh	m		per veh	km/h			
South	: Argyle St (S)												
1	L2	15	2.0	0.467	22.1	LOS B	15.1	110.0	0.65	0.58	35.0			
2	T1	517	5.0	0.467	16.2	LOS B	15.1	110.0	0.65	0.60	34.5			
3	R2	166	2.0	0.467	26.4	LOS B	8.1	57.8	0.71	0.75	31.1			
Appro	ach	698	4.2	0.467	18.7	LOS B	15.1	110.0	0.67	0.64	33.6			
East:	Margaret St	(E)												
4	L2	140	2.0	0.455	52.9	LOS D	7.0	49.7	0.96	0.79	22.0			
5	T1	5	2.0	0.330	45.4	LOS D	5.0	35.4	0.93	0.77	22.2			
6	R2	97	2.0	0.330	51.9	LOS D	5.0	35.4	0.93	0.77	22.2			
Appro	ach	242	2.0	0.455	52.4	LOS D	7.0	49.7	0.95	0.79	22.1			
North:	: Argyle St (N	۷)												
7	L2	75	2.0	0.210	19.0	LOS B	5.6	40.4	0.53	0.57	36.1			
8	T1	333	5.0	0.210	13.5	LOS A	5.6	40.4	0.54	0.52	36.1			
9	R2	5	2.0	0.210	20.5	LOS B	5.5	40.1	0.56	0.48	36.1			
Appro	ach	413	4.4	0.210	14.6	LOS B	5.6	40.4	0.54	0.53	36.1			
West:	Cliffe St (W))												
10	L2	10	2.0	0.058	51.4	LOS D	0.7	5.0	0.90	0.68	9.6			
11	T1	5	2.0	0.058	45.0	LOS D	0.7	5.0	0.90	0.68	9.6			

12	R2	11	2.0	0.043	51.4	LOS D	0.5	3.7	0.90	0.68	9.4
Appro	ach	26	2.0	0.058	50.2	LOS D	0.7	5.0	0.90	0.68	9.5
All Vel	hicles	1379	3.8	0.467	24.0	LOS B	15.1	110.0	0.68	0.63	31.0

Move	ement Per	formance	- Vehic	les							
Mov II	D ODMo	Demand	Flows D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East:	Argyle Stree	et									
5	T1	198	5.0	0.193	7.0	LOS A	1.1	8.3	0.38	1.10	48.7
6	R2	18	2.0	0.193	11.8	LOS B	1.1	8.3	0.38	1.10	48.7
Appro	ach	216	4.8	0.193	7.4	LOS A	1.1	8.3	0.38	0.55	48.7
North:	Regreme F	Road									
7	L2	74	2.0	0.241	9.7	LOS A	1.3	9.6	0.58	1.51	45.0
9	R2	146	2.0	0.241	13.6	LOS B	1.3	9.6	0.58	1.51	45.0
Appro	ach	220	2.0	0.241	12.3	LOS B	1.3	9.6	0.58	0.76	45.0
West:	Argyle Stre	et									
10	L2	121	2.0	0.350	7.1	LOS A	2.5	18.3	0.13	0.99	50.3
11	T1	418	5.0	0.350	6.2	LOS A	2.5	18.3	0.13	0.99	50.3
Appro	ach	539	4.3	0.350	6.4	LOS A	2.5	18.3	0.13	0.49	50.3
All Ve	hicles	975	3.9	0.350	8.0	LOS A	2.5	18.3	0.28	0.57	48.6



Move	ement Per	formance	- Vehic	cles							
Mov I	D ODMo	Demand	Flows [Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East:	Menangle S	Street									
5	T1	244	5.0	0.129	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
6	R2	25	0.0	0.026	11.0	LOS B	0.1	0.7	0.55	0.73	46.0
Appro	Approach 269		4.5	0.129	1.0	NA	0.1	0.7	0.05	0.07	58.3
North	: Site Acces	S									
7	L2	2	0.0	0.034	11.7	LOS B	0.1	0.8	0.56	0.80	45.3
9	R2	22	0.0	0.034	11.8	LOS B	0.1	0.8	0.56	0.80	45.3
Appro	ach	24	0.0	0.034	11.8	LOS B	0.1	0.8	0.56	0.80	45.3
West:	Menangle	Street									
10	L2	67	0.0	0.036	8.2	LOS A	0.0	0.0	0.00	0.67	48.9
11	T1	545	5.0	0.289	0.0	LOS A	0.0	0.0	0.00	0.00	59.9
Appro	ach	612	4.5	0.289	0.9	NA	0.0	0.0	0.00	0.07	58.5
All Ve	hicles	905	4.4	0.289	1.3	NA	0.1	0.8	0.03	0.09	58.0

PM PEAK

Argyle St/Margaret St/Cliffe St

Movement Performance - Vehicles Mov ID ODMo Demand Flows Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average											
Mov I	D ODMo	Demand	Flows D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Argyle St	(S)									
1	L2	5	2.0	0.346	24.1	LOS B	10.1	73.9	0.65	0.56	33.9
2	T1	311	5.0	0.346	17.7	LOS B	10.1	73.9	0.65	0.56	33.9
3	R2	124	2.0	0.450	33.8	LOS C	5.1	36.3	0.78	0.79	27.6
Appro	ach	440	4.1	0.450	22.3	LOS B	10.1	73.9	0.69	0.63	31.8
East:	Margaret S	t (E)									
4	L2	190	2.0	0.456	47.8	LOS D	9.0	64.2	0.92	0.81	23.2
5	T1	5	2.0	0.215	39.0	LOS C	4.0	28.6	0.87	0.76	23.9
6	R2	85	2.0	0.215	45.5	LOS D	4.0	28.6	0.87	0.76	23.9
Appro	ach	280	2.0	0.456	46.9	LOS D	9.0	64.2	0.91	0.79	23.4
North	: Argyle St	(N)									
7	L2	102	2.0	0.347	24.1	LOS B	10.2	74.0	0.65	0.65	33.1
8	T1	509	5.0	0.347	18.4	LOS B	10.2	74.0	0.66	0.61	33.1
9	R2	9	2.0	0.347	25.3	LOS B	10.0	72.6	0.66	0.58	33.1
Appro	ach	620	4.5	0.347	19.4	LOS B	10.2	74.0	0.66	0.62	33.1
West:	Cliffe St (V	V)									
10	L2	5	2.0	0.038	51.1	LOS D	0.5	3.3	0.89	0.65	9.7
11	T1	5	2.0	0.038	44.7	LOS D	0.5	3.3	0.89	0.65	9.7
12	R2	5	2.0	0.020	51.0	LOS D	0.2	1.7	0.89	0.65	9.4
Appro	ach	15	2.0	0.038	48.9	LOS D	0.5	3.3	0.89	0.65	9.6
All Ve	hicles	1355	3.8	0.456	26.4	LOS B	10.2	74.0	0.72	0.66	29.9

Argyle St/Menangle St (Proposed Signalised Layout)

Move	ment Per	formance	- Vehic	cles							
	ODMo			Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
IVIOV IL	V - V	Total	HV	ocy. Odin	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Argyle St	(S)									
1	L2	7	2.0	0.427	26.2	LOS B	13.4	98.0	0.70	0.62	32.6
2	T1	381	5.0	0.427	19.8	LOS B	13.4	98.0	0.70	0.62	32.6
3	R2	192	2.0	0.629	46.0	LOS D	9.6	68.2	0.99	0.90	23.0
Approa	ach	580	4.0	0.629	28.5	LOS C	13.4	98.0	0.79	0.71	28.8
East: N	Menangle S	St (E)									
4	L2	437	2.0	0.584	32.3	LOS C	18.3	130.6	0.83	0.82	25.0
5	T1	5	2.0	0.584	27.0	LOS B	18.3	130.6	0.83	0.82	25.0
6	R2	77	2.0	0.149	27.5	LOS B	2.6	18.5	0.66	0.73	26.5
Approa	ach	519	2.0	0.584	31.6	LOS C	18.3	130.6	0.81	0.81	25.2
North:	Argyle St (N)									
7	L2	74	2.0	0.598	38.3	LOS C	16.9	122.4	0.89	0.79	26.6
8	T1	666	5.0	0.598	32.7	LOS C	16.9	122.4	0.89	0.78	26.8
9	R2	5	2.0	0.598	39.1	LOS C	16.4	119.8	0.89	0.77	26.9
Approa	ach	745	4.7	0.598	33.3	LOS C	16.9	122.4	0.89	0.78	26.8
West:	Menangle :	St (W)									
10	L2	5	2.0	0.013	26.7	LOS B	0.3	2.2	0.61	0.59	22.5
11	T1	5	2.0	0.013	21.5	LOS B	0.3	2.2	0.61	0.59	22.5
12	R2	8	2.0	0.038	43.0	LOS D	0.3	2.4	0.80	0.69	16.5
Approa	ach	18	2.0	0.038	32.5	LOS C	0.3	2.4	0.70	0.63	19.3
All Veh	nicles	1862	3.7	0.629	31.3	LOS C	18.3	130.6	0.83	0.77	26.8

Move	ment Per	formance	- Vehic	eles							
Mov ID	ODMo	Demand	I Flows D	Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: A	Argyle Stree	et	,		,			,			
5	T1	449	5.0	0.450	7.2	LOS A	3.4	24.6	0.47	1.17	47.9
6	R2	86	2.0	0.450	12.1	LOS B	3.4	24.6	0.47	1.17	47.9
Approa	Approach		4.5	0.450	8.0	LOS A	3.4	24.6	0.47	0.59	47.9
North: Regreme R		Road									
7	L2	21	2.0	0.160	8.2	LOS A	0.9	6.2	0.42	1.36	45.3
9	R2	150	2.0	0.160	12.2	LOS B	0.9	6.2	0.42	1.36	45.3
Approa	ach	171	2.0	0.160	11.7	LOS B	0.9	6.2	0.42	0.68	45.3
West:	Argyle Stre	et									
10	L2	166	2.0	0.294	7.5	LOS A	1.9	14.0	0.30	1.05	49.1
11	T1	212	5.0	0.294	6.6	LOS A	1.9	14.0	0.30	1.05	49.1
Approa	ach	378	3.7	0.294	7.0	LOS A	1.9	14.0	0.30	0.53	49.1
All Veh	nicles	1084	3.8	0.450	8.2	LOS A	3.4	24.6	0.40	0.58	47.9

Move	ment Per	formance	- Vehic	cles							
Mov ID	ODMo	Demand	Flows D	Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: N	Menangle S	Street									
5	T1	462	5.0	0.245	0.0	LOS A	0.0	0.0	0.00	0.00	59.9
6	R2	92	0.0	0.071	9.7	LOS A	0.3	2.2	0.42	0.68	47.0
Approa	ach	554	4.2	0.245	1.6	NA	0.3	2.2	0.07	0.11	57.3
North: Site Access		S									
7	L2	37	0.0	0.140	11.1	LOS B	0.5	3.5	0.46	0.78	45.8
9	R2	73	0.0	0.140	11.2	LOS B	0.5	3.5	0.46	0.78	45.8
Approa	ach	110	0.0	0.140	11.2	LOS B	0.5	3.5	0.46	0.78	45.8
West:	Menangle S	Street									
10	L2	89	0.0	0.048	8.2	LOS A	0.0	0.0	0.00	0.67	48.9
11	T1	258	5.0	0.137	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ach	347	3.7	0.137	2.1	NA	0.0	0.0	0.00	0.17	56.7
All Veh	nicles	1011	3.6	0.245	2.8	NA	0.5	3.5	0.09	0.21	55.6

BASE plus DEVELOPMENT SCENARIO – 2026

AM PEAK

Argyle St/Margaret St/Cliffe St

Movement Performance - Vehicles Mov ID ODMo Demand Flows Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average												
Mov II	ODMo	Demand	Flows D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average	
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed	
		veh/h	%	v/c	sec		veh	m		per veh	km/h	
South	: Argyle St	(S)										
1	L2	21	2.0	0.696	24.2	LOS B	27.5	200.5	0.77	0.71	33.6	
2	T1	708	5.0	0.696	17.8	LOS B	27.5	200.5	0.77	0.71	33.6	
3	R2	238	2.0	0.700	35.6	LOS C	11.1	78.9	0.86	0.87	27.0	
Appro	ach	967	4.2	0.700	22.3	LOS B	27.5	200.5	0.79	0.75	31.7	
East: I	Margaret S	t (E)										
4	L2	184	2.0	0.677	57.2	LOS E	9.8	69.8	1.00	0.84	21.0	
5	T1	5	2.0	0.452	48.3	LOS D	6.2	44.4	0.97	0.79	21.6	
6	R2	118	2.0	0.452	54.8	LOS D	6.2	44.4	0.97	0.79	21.6	
Appro	ach	307	2.0	0.677	56.1	LOS D	9.8	69.8	0.99	0.82	21.2	
North:	Argyle St (
7	L2	91	2.0	0.267	19.0	LOS B	7.5	54.3	0.54	0.58	36.2	
8	T1	423	5.0	0.267	14.6	LOS B	7.5	54.3	0.58	0.55	35.3	
9	R2	7	2.0	0.267	22.7	LOS B	7.2	52.6	0.61	0.53	34.7	
Appro	ach	521	4.4	0.267	15.5	LOS B	7.5	54.3	0.57	0.56	35.5	
West:	Cliffe St (V	/)										
10	L2	11	2.0	0.062	51.4	LOS D	0.8	5.4	0.90	0.68	9.6	
11	T1	5	2.0	0.062	45.0	LOS D	0.8	5.4	0.90	0.68	9.6	
12	R2	16	2.0	0.063	51.6	LOS D	0.8	5.4	0.90	0.70	9.3	
Appro	ach	32	2.0	0.063	50.5	LOS D	0.8	5.4	0.90	0.69	9.4	
All Ve	hicles	1827	3.9	0.700	26.5	LOS B	27.5	200.5	0.76	0.70	29.7	

Move	ment Per	formance	- Vehi	cles							
Mov IE	ODMo	Demand	Flows	Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Argyle St	(S)									
1	L2	35	2.0	1.328	604.7	LOS F	523.9	3783.9	1.00	7.59	3.2
2	T1	928	5.0	1.328	598.3	LOS F	523.9	3783.9	1.00	7.59	3.2
3	R2	676	2.0	1.328	604.1	LOS F	523.9	3783.9	1.00	7.59	3.2
Approa	ach	1639	3.7	1.328	600.8	NA	523.9	3783.9	1.00	7.59	3.2
East: N	Menangle S	St (E)									
4	L2	155	2.0	3.100	3847.8	LOS F	240.9	1715.2	1.00	13.97	0.5
5	T1	12	2.0	3.100	3846.4	LOS F	240.9	1715.2	1.00	13.97	0.5
6	R2	109	2.0	3.100	3847.9	LOS F	240.9	1715.2	1.00	13.97	0.5
Approa	ach	276	2.0	3.100	3847.8	LOS F	240.9	1715.2	1.00	13.97	0.5
North:	Argyle St (N)									
7	L2	118	2.0	0.367	18.3	LOS B	7.5	54.8	1.00	0.02	35.8
8	T1	545	5.0	0.367	12.7	LOS A	7.5	54.8	1.00	0.02	35.8
9	R2	9	2.0	0.367	19.4	LOS B	7.5	54.8	1.00	0.02	35.8
Approa	ach	672	4.4	0.367	13.7	NA	7.5	54.8	1.00	0.02	35.8
West:	Menangle :	St (W)									
10	L2	10	2.0	1.000	388.3	LOS F	8.5	60.8	1.00	1.38	2.5

11	T1	15	2.0	1.000	386.3	LOS F	8.5	60.8	1.00	1.38	2.5
12	R2	26	2.0	1.000	388.8	LOS F	8.5	60.8	1.00	1.38	2.5
Appro	ach	51	2.0	1.000	388.0	LOS F	8.5	60.8	1.00	1.38	2.5
All Ve	hicles	2638	3.7	3.100	786.9	NA	523.9	3783.9	1.00	6.21	2.4

Move	ment Per	formance	- Vehic	cles							
Mov II	O ODMo	Demand	Flows [Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	: Argyle St	(S)									
1	L2	35	2.0	0.685	14.1	LOS A	26.4	192.5	0.57	0.54	40.6
2	T1	928	5.0	0.685	7.7	LOS A	26.4	192.5	0.57	0.54	40.6
3	R2	676	2.0	0.815	37.2	LOS C	27.2	193.4	0.93	1.08	25.6
Appro	ach	1639	3.7	0.815	20.0	LOS B	27.2	193.4	0.72	0.76	32.9
East: I	Menangle S	St (E)									
4	L2	155	2.0	0.618	54.3	LOS D	8.7	61.9	0.99	0.81	19.8
5	T1	12	2.0	0.618	49.0	LOS D	8.7	61.9	0.99	0.81	19.8
6	R2	109	2.0	0.559	55.5	LOS D	5.7	40.8	0.98	0.80	19.5
Appro	ach	276	2.0	0.618	54.6	LOS D	8.7	61.9	0.99	0.81	19.7
North:	Argyle St ((N)									
7	L2	118	2.0	0.658	42.0	LOS C	17.4	125.8	0.93	0.82	25.2
8	T1	545	5.0	0.658	38.7	LOS C	17.4	125.8	0.94	0.82	24.7
9	R2	9	2.0	0.658	47.1	LOS D	14.8	107.9	0.96	0.82	24.3
Appro	ach	672	4.4	0.658	39.4	LOS C	17.4	125.8	0.94	0.82	24.8
West:	Menangle	St (W)									
10	L2	10	2.0	0.092	50.8	LOS D	1.2	8.4	0.90	0.69	14.6
11	T1	15	2.0	0.092	45.6	LOS D	1.2	8.4	0.90	0.69	14.6
12	R2	26	2.0	0.248	62.8	LOS E	1.4	10.1	0.98	0.72	12.6
Appro	ach	51	2.0	0.248	55.4	LOS D	1.4	10.1	0.94	0.70	13.5
All Vel	hicles	2638	3.7	0.815	29.2	LOS C	27.2	193.4	0.81	0.78	28.1

Movement Performance - Vehicles												
		Tormance	- venic	ies								
Mov	ID ODMo	Demand	Flows D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average	
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed	
		veh/h	%	v/c	sec		veh	m		per veh	km/h	
East:	Menangle S	Street										
5	T1	334	5.0	0.177	0.0	LOS A	0.0	0.0	0.00	0.00	60.0	
6	R2	26	0.0	0.038	12.8	LOS B	0.1	1.0	0.63	0.82	44.3	
Appro	oach	360	4.6	0.177	0.9	NA	0.1	1.0	0.05	0.06	58.5	
North	n: Site Acces	S										
7	L2	2	0.0	0.052	14.6	LOS B	0.2	1.2	0.71	0.91	42.6	
9	R2	22	0.0	0.052	14.7	LOS B	0.2	1.2	0.71	0.91	42.6	
Appro	oach	24	0.0	0.052	14.7	LOS B	0.2	1.2	0.71	0.91	42.6	
West	:: Menangle \$	Street										
10	L2	65	0.0	0.035	8.2	LOS A	0.0	0.0	0.00	0.67	48.9	
11	T1	765	5.0	0.405	0.1	LOS A	0.0	0.0	0.00	0.00	59.9	
Approach 830 4.6 0.405		0.405	0.7	NA	0.0	0.0	0.00	0.05	58.9			
All Ve	ehicles	1214	4.5	0.405	1.1	NA	0.2	1.2	0.03	0.07	58.3	

PM PEAK

Argyle St/Margaret St/Cliffe St

	J /	8		,									
Movement Performance - Vehicles													
Mov II	D ODMo	Demand	Flows [Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average		
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed		
		veh/h	%	v/c	sec		veh	m		per veh	km/h		
South	: Argyle St	(S)											
1	L2	5	2.0	0.408	26.9	LOS B	14.1	102.9	0.67	0.59	32.3		
2	T1	371	5.0	0.408	20.5	LOS B	14.1	102.9	0.67	0.59	32.3		
3	R2	175	2.0	0.896	83.4	LOS F	14.5	103.1	1.00	1.15	16.6		
Appro	ach	551	4.0	0.896	40.6	LOS C	14.5	103.1	0.77	0.77	24.9		
East:	Margaret St	t (E)											
4	L2	274	2.0	0.908	73.2	LOS F	18.8	133.9	0.94	1.03	18.1		
5	T1	5	2.0	0.223	40.9	LOS C	5.2	37.2	0.84	0.77	23.4		
6	R2	102	2.0	0.223	47.4	LOS D	5.2	37.2	0.84	0.77	23.4		
Appro	ach	381	2.0	0.908	65.9	LOS E	18.8	133.9	0.91	0.95	19.3		
North:	: Argyle St (N)											
7	L2	117	2.0	0.469	27.8	LOS B	17.0	123.0	0.70	0.68	31.3		
8	T1	718	5.0	0.469	22.0	LOS B	17.0	123.0	0.71	0.66	31.3		
9	R2	10	2.0	0.469	29.0	LOS C	16.4	119.8	0.71	0.64	31.2		
Appro	ach	845	4.5	0.469	22.9	LOS B	17.0	123.0	0.71	0.66	31.3		
West:	Cliffe St (W	/)											
10	L2	5	2.0	0.043	59.1	LOS E	0.5	3.9	0.91	0.66	8.6		
11	T1	5	2.0	0.043	52.7	LOS D	0.5	3.9	0.91	0.66	8.6		
12	R2	8	2.0	0.036	59.2	LOS E	0.4	3.1	0.91	0.67	8.3		
Appro		18	2.0	0.043	57.4	LOS E	0.5	3.9	0.91	0.66	8.5		
All Ve	hicles	1795	3.8	0.908	37.8	LOS C	18.8	133.9	0.77	0.76	25.6		

Movement Performance - Vehicles													
	D ODMo v			eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed		
		veh/h	%	v/c	sec		veh	m		per veh	km/h		
South	: Argyle St	(S)											
1	L2	7	2.0	0.448	20.7	LOS B	14.7	107.3	0.62	0.55	35.9		
2	T1	477	5.0	0.448	14.3	LOS A	14.7	107.3	0.62	0.55	35.9		
3	R2	221	2.0	0.708	51.1	LOS D	11.4	81.2	1.00	1.03	21.7		
Appro	ach	705	4.0	0.708	25.9	LOS B	14.7	107.3	0.74	0.70	29.9		
East:	Menangle S	St (E)											
4	L2	584	2.0	0.670	28.6	LOS C	24.0	171.1	0.82	0.84	26.1		
5	T1	5	2.0	0.670	23.3	LOS B	24.0	171.1	0.82	0.84	26.1		
6	R2	103	2.0	0.252	35.7	LOS C	4.1	29.5	0.78	0.76	24.0		
Appro	ach	692	2.0	0.670	29.6	LOS C	24.0	171.1	0.82	0.83	25.8		
North:	Argyle St (N)											
7	L2	101	2.0	0.697	34.5	LOS C	23.7	172.0	0.89	0.81	28.0		
8	T1	954	5.0	0.697	28.8	LOS C	23.7	172.0	0.89	0.80	28.2		
9	R2	5	2.0	0.697	35.3	LOS C	23.2	169.4	0.89	0.79	28.4		
Appro	ach	1060	4.7	0.697	29.4	LOS C	23.7	172.0	0.89	0.80	28.2		
West:	Menangle	St (W)											
10	L2	5	2.0	0.017	33.5	LOS C	0.4	2.6	0.71	0.62	19.5		
11	T1	5	2.0	0.017	28.4	LOS B	0.4	2.6	0.71	0.62	19.5		
12	R2	9	2.0	0.049	42.5	LOS D	0.4	2.7	0.80	0.69	16.6		
Appro	ach	19	2.0	0.049	36.4	LOS C	0.4	2.7	0.75	0.65	18.0		
All Ve	hicles	2476	3.7	0.708	28.5	LOS C	24.0	172.0	0.82	0.78	27.9		

Move	Movement Performance - Vehicles														
Mov II	O ODMo	Demand	Flows	Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average				
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed				
		veh/h	%	v/c	sec		veh	m		per veh	km/h				
East:	Argyle Stree	et													
5	T1	637	5.0	0.638	7.9	LOS A	6.1	44.6	0.64	1.26	47.0				
6	R2	107	2.0	0.638	12.7	LOS B	6.1	44.6	0.64	1.26	47.0				
Appro	Approach		4.6	0.638	8.6	LOS A	6.1	44.6	0.64	0.63	47.0				
North: Regreme Road		Road													
7	L2	26	2.0	0.200	8.5	LOS A	1.1	8.0	0.47	1.40	45.1				
9	R2	179	2.0	0.200	12.5	LOS B	1.1	8.0	0.47	1.40	45.1				
Appro	ach	205	2.0	0.200	12.0	LOS B	1.1	8.0	0.47	0.70	45.1				
West:	Argyle Stre	et													
10	L2	198	2.0	0.360	7.7	LOS A	2.6	19.0	0.38	1.08	48.7				
11	T1	251	5.0	0.360	6.8	LOS A	2.6	19.0	0.38	1.08	48.7				
Approach		449	3.7	0.360	7.2	LOS A	2.6	19.0	0.38	0.54	48.7				
All Ve	hicles	1398	3.9	0.638	8.6	LOS A	6.1	44.6	0.53	0.61	47.2				

Movement Performance - Vehicles													
Mov ID	ODMo	Demand	Flows	Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average		
		Total	HV		Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed		
		veh/h	%	v/c	sec		veh	m		per veh	km/h		
East: N	Menangle S	treet			,			,					
5	T1	653	5.0	0.346	0.1	LOS A	0.0	0.0	0.00	0.00	59.9		
6	R2	88	0.0	0.076	10.1	LOS B	0.3	2.3	0.48	0.72	46.8		
Approa	ach	741	4.4	0.346	1.2	NA	0.3	2.3	0.06	0.09	58.0		
North:	North: Site Access												
7	L2	35	0.0	0.182	13.2	LOS B	0.6	4.4	0.59	0.84	43.9		
9	R2	70	0.0	0.182	13.3	LOS B	0.6	4.4	0.59	0.84	43.9		
Approa	ach	105	0.0	0.182	13.3	LOS B	0.6	4.4	0.59	0.84	43.9		
West:	Menangle S	Street											
10	L2	82	0.0	0.044	8.2	LOS A	0.0	0.0	0.00	0.67	48.9		
11	T1	362	5.0	0.192	0.0	LOS A	0.0	0.0	0.00	0.00	60.0		
Approach		444	4.1	0.192	1.5	NA	0.0	0.0	0.00	0.12	57.6		
All Vel	nicles	1290	3.9	0.346	2.3	NA	0.6	4.4	0.08	0.16	56.4		